VALUE ENGINEERING SUMMARY OF US 119/ZEBULON TO BENT MT. WPI NO. 12-308.1 & 308.2 PIKE COUNTY, KENTUCKY

JANUARY 6-14, 1997

Prepared by: Ventry Engineering

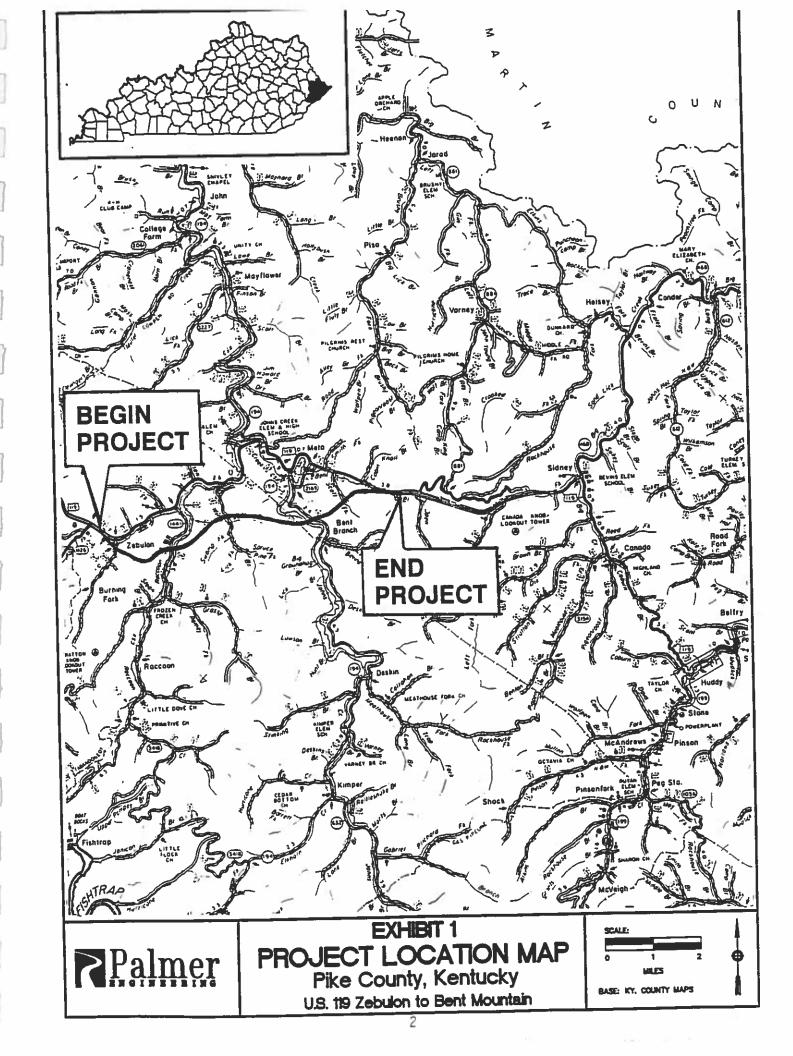
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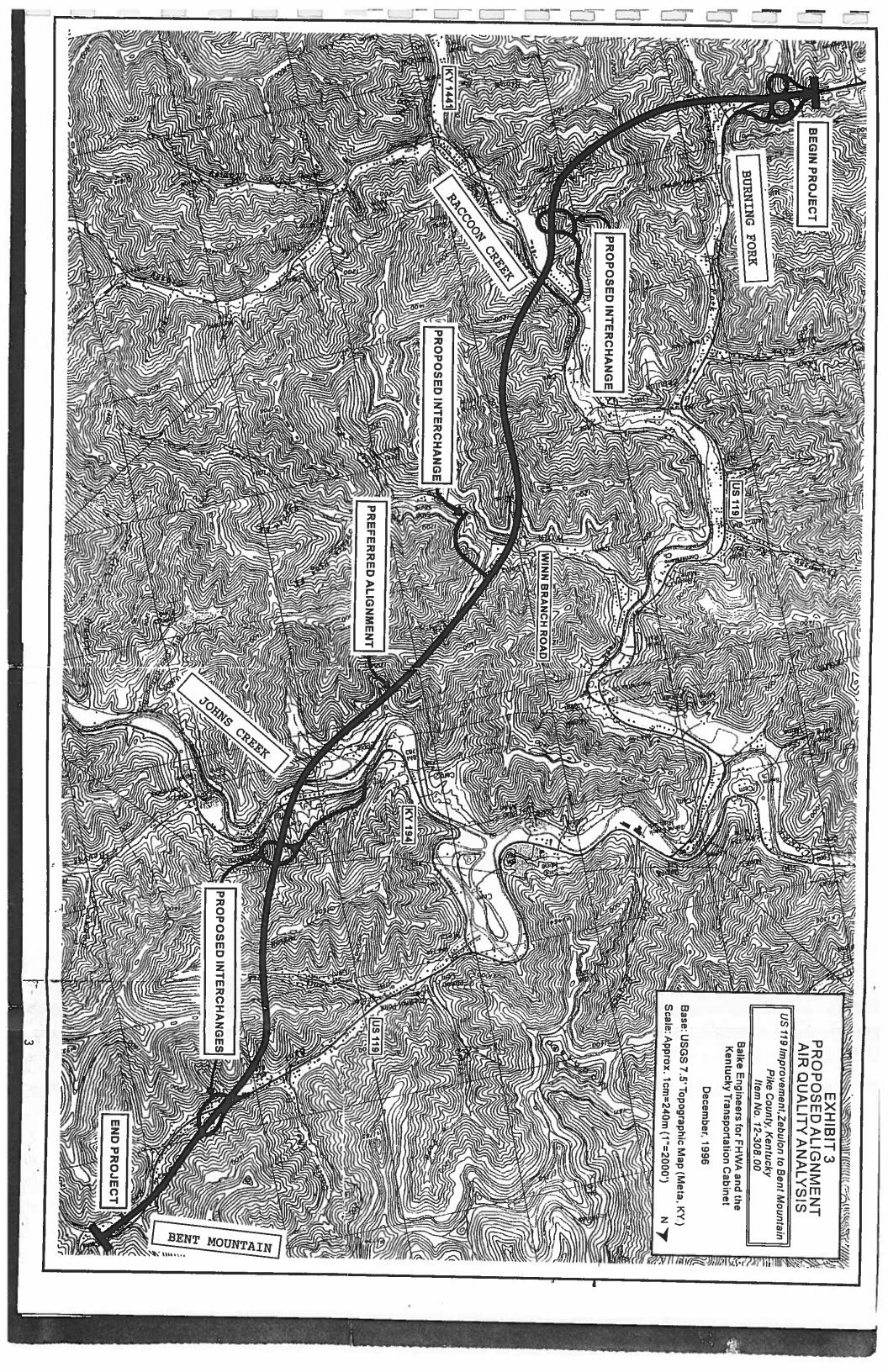
Kentucky Transportation Cabinet, Department of Highways

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I. LOCATION OF PROJECT





II. TEAM MEMBERS AND PROJECT DESCRIPTION

TEAM MEMBERS

NAME	AFFILIATION	EXPERTISE	PHONE
Jack Trickey, P.E. C.V.S.	Ventry Engineering	Team Leader	904/627-3900
Don Keenan, P.E.	Ventry Engineering	Structural Team Member	904/627-3900
Ron Whichel, P.E.	Ventry Engineering	Cost Estimating Team Member	904/627-3900
Dallas Gray	Ventry Engineering	Right of Way Team Member	904/627-3900
Ken Sperry, P.E.	KY Transportation Cabinet	Highway Design	502/564-3280
Steve Halloran, P.E.	KY Transportation Cabinet	Construction	502/564-4780
Daryl Greer, P.E.	KY Transportation Cabinet	Value Engineering	502/564-3280
Jeff Jasper, E.I.T.	KY Transportation Cabinet	Highway Design	502/564-3280

PROJECT DESCRIPTION

The project provides for the relocation of 14.3 kilometers (8.9 miles) of U.S. 119 in Pike County, Kentucky. The proposed new alignment is approximately 12 kilometers (7.5 miles) in length. The project relocates existing U.S. 119 from Burning Fork Road to near Bent Mountain.

Four new mainline bridges cross Burning Fork Road, Racoon Creek, Johns Creek and existing U.S. 119. Winn Branch and Scott Fork are crossed with culverts with mainline access provided only to the southeast portion of Winn Branch. Overpasses are called for on the approaches at Raccoon Creek and Johns Creek to eliminate left turning vehicles across the median.

The project is functionally classified as a rural arterial in mountainous terrain.

The proposed typical section provides a 12 meter depressed median with two 7.2 meter roadways and 3.6 meter outside shoulders, with 3.0 meters paved.

The proposed project will displace approximately 116 families, 5 businesses and 213 graves.

This existing and proposed facility serves local traffic, major coal operations and is a major intrastate route.

COST ESTIMATE

Roadway Excavation Drainage Pavement and Base Bridges Compression Station Miscellaneous (Silt checks, Guar Staking, R/W Fen	se on drail, End treatme		\$ 64,623,000 3,129,000 8,241,000 47,459,000 6,500,000
Staking, K/W Ten	ce, Italiic, water	,	1,164,000
Mobilization	3.0%		3,933,480
Demobilization	1.5%	Subtotal	1,966,740 \$137,016,220
Eng. & Conting.	20%		27,403,244
(Approaches)			0.000.000
Burning Fork Racoon Branch			9,202,000 8,491,000
Winn Branch			3,113,000
Johns Creek			7,853,000
Bent Mountain			5,147,000
		Subtotal	\$ 33,806,000
		Total Construction	\$198,225,464
Right of Way			\$ 32,379,000
Utility Relocation			<u>5,570,000</u>
	W	Total Project Estimate	<u>\$236,174,464</u>

US 119 Pike Co., 1996

Figure 3 Typical Sections US 119 Corridor

IOCK CUT -0.6 m W/ G.R. AS REO'D.

12.6 ш

0.50 E

12.6 m

YBYRUZ 3

TYPICAL DECK SECTION

TYPICAL SECTION WITH 12 m DEPRESSED MEDIAN

NOT TO SCALE

TYPICAL SECTIONS

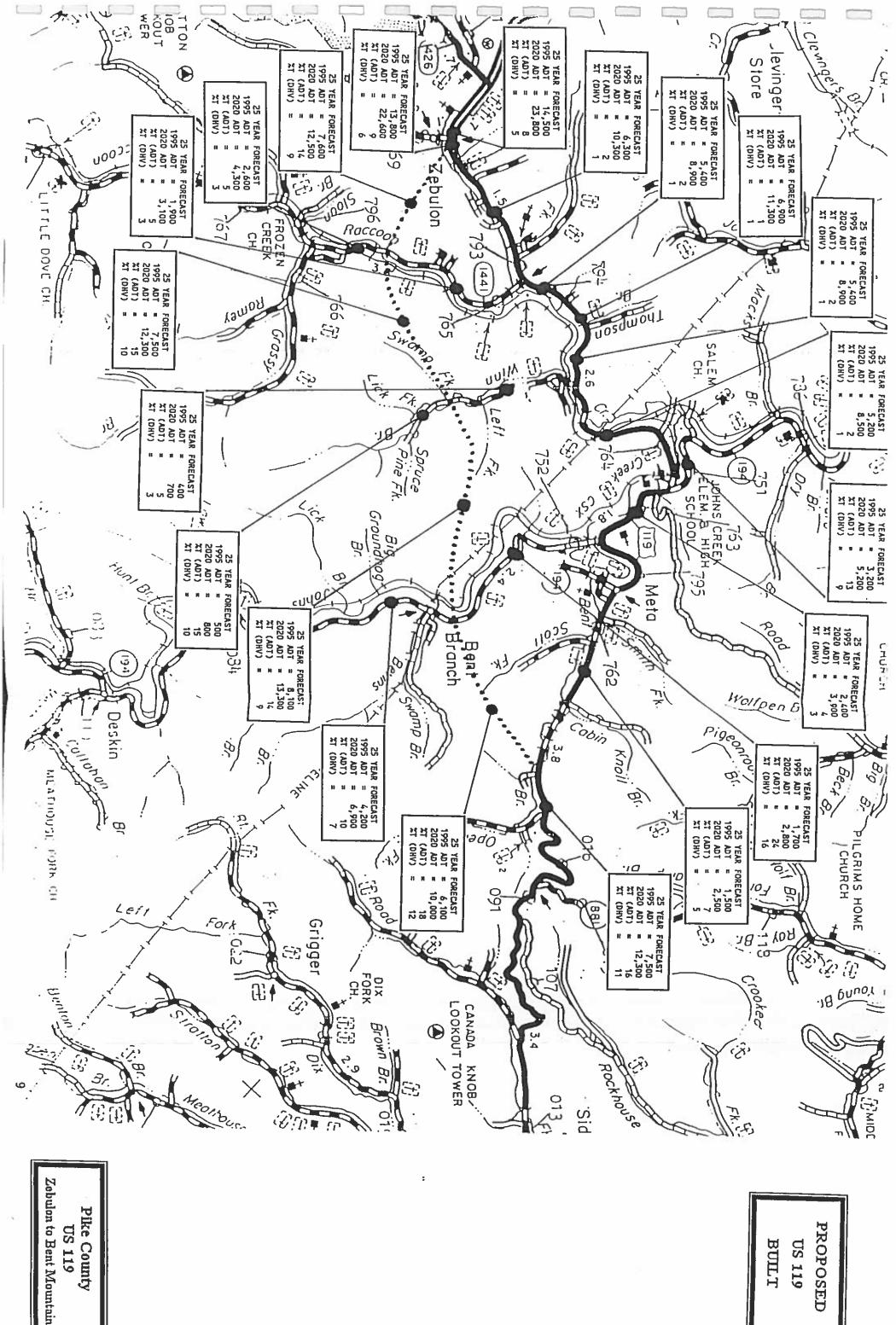
US 119 ZEBULDN TO BENT MOUNTAIN PROJECT PLANNING REPORT

8

ROCK CUT

0.6 m W/ G.R.-AS RE0'0.

1:4 DESIGNBLE



Pike County US 119

III. INVESTIGATION PHASE

US 119/ZEBULON TO BENT MOUNTAIN V.E. STUDY BRIEFING January 7, 1997

NAME	AFFILIATION	PHONE
NAME	AFFILIATION	FHUNE
Jack Trickey	Ventry Engineering	904/627-3900
Don Keenan	Ventry Engineering	904/627-3900
Steve Halloran	KTC Construction	502/564-4780
Steve Hoefler	KTC Highway Design	502/564-3280
Randy Stephens	Palmer Engineering	606/744-1218
David Lindeman	Palmer Engineering	606/744-1218
Charles Reichenbach	KY D.O.H. Dist. #12	606/433-7791
Denton Biliter	Chief Dist. Eng., KY D.O.H. Dist. #12	606/433-7791
James D. Wright	Dist. Const. Eng., KY D.O.H. Dist. #12	606/433-7791
Keith R. Damron	Dist. #12 Design Engineer	606/433-7791
Robin R. Justice	Dist. #12 Design EIT	606/433-7791
Dallas Gray	Ventry Engineering	904/627-3900
Ron Whichel	Ventry Engineering	904/627-3900
Jeff Jasper	KTC Highway Design	502/564-3280
Ken Sperry	KTC Highway Design	502/564-3280
Daryl Greer	KTC Value Engineer	502/564-3280
Janet R. Coffey	KTC Dist. #12 Operations	502/564-4556
Dexter Newman	KTC Dist. #12 Const.	606/433-7791

PERSONS CONTACTED

NAME	AFFILIATION	PHONE
Randy Stephens	Palmer Engineering Co.	606/744-1218
Robert Miller	Tensar, Atlanta, Ga.	800/292-4459
Jerry Justice	Dist. #12, R/W	606/433-7765
Joe Emberson	Tensar, Atlanta, Ga.	800/292-4459

INVESTIGATION

The following have been identified by the Value Engineering Team as areas of focus and investigation for the Value Engineering process:

Areas identified as high cost items during the investigation phase:

<u>ITEM</u>	COST	FUNCTION
EXCAVATION	\$64,600,000	ESTABLISH PROFILE
DRAINAGE	\$3,100,000	CONVEY WATER
PAV'T & BASE	\$8,200,000	SUPPORT VEHICLES
RIGHT OF WAY	\$25,000,000	PROVIDE LAND
STRUCTURES	\$47,500,000	SEPARATE TRAFFIC
APPROACHES	\$33,800,000	PROVIDE ACCESS

FUNCTIONAL ANALYSIS WORKSHEET, INFORMATION PHASE

PROJECT: US 119/ZEBULON TO BENT MOUNTAIN

DATE: JANUARY 6-14, 1997

ITEM	FUNCT. VERB	FUNCT.	ТҮРЕ	COST	WORTH	VALUE INDEX
EXCAVATION	establish establish accom. facilitate facilitate	profile align. typical access develop.	B B B S	\$64,600,000	\$60,000,000	1.1
DRAINAGE	convey minimize	water erosion	B B	\$3,100,000	\$3,100,000	1.0
PAV'T AND BASE	support support protect remove increase reduce	vehicles loads base water traction rutting	B B B B	\$8,200,000	\$8,200,000	1.0
RIGHT OF WAY	provide accom.	land design	B B	\$25,000,000	\$23,000,000	1.1
STRUCTURES	span separate convey	creek traffic water	B B B	\$47,500,000	\$44,000,000	1.1
BURNING FORK APPROACH	provide eliminate	access left turns	B S	\$9,200,000	\$8,200,000	1.1
RACCOON CREEK APPROACH	provide eliminate separate	access left turns traffic	B S S	\$8,500,000	\$8,000,000	1.1
WINN BRANCH APPROACH	provide	access	S	\$3,100,000	\$3,100,000	1.0
JOHNS CREEK APPROACH	provide eliminate separate	access left turns traffic	B S S	\$7,900,000	\$7,000.000	1.3
BENT MOUNTAIN APPROACH	provide eliminate	access left turns	BS	\$5,100,000	\$5,100,000	1.0

IV. SPECULATION PHASE

SPECULATION

Ideas generated, utilizing the brainstorming method, for performing the functions of previously identified areas of focus.

EXCAVATION

- Revise the median width from a 12 m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach
- Revise the alignment between station 505 + 800 and station 507 + 300 to turn easterly along the hollow then back to the proposed alignment
- Increase the grade between station 508 + 600 and station 509 + 750
- Bifurcate the roadways in fill areas

PAVEMENT AND BASE

- Use-concrete-pavement instead of asphalt-pavement for the US-119-mainline roadways
- Construct the outside lane to be 4.3m (14') wide and strip the edge line at 3.6m (12') to provide an additional 0.6m (2') of full depth roadway for edge of pavement support
- Construct a full depth shoulder to reduce future maintenance cost caused by heavy trucks

STRUCTURES

• Revise the bridge typical section to only provide a 3.0m outside shoulder instead of the 3.6m shoulder proposed

US 119 AT BURNING FORK APPROACH

- Eliminate Ramp D from station 40 + 000 to station 40 + 535 and utilize Ramp E with a 15m radius turnlane to provide the WB to NB movement
- Revise the north side of the intersection to reflect a half diamond type interchange with the long radius currently proposed for the SB to WB movement for the heavy trucks and retaining the relocated US 119 configuration currently proposed for the south side of the intersection

RACCOON CREEK APPROACH

 Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp A and left of mainline station 502+900

WINN BRANCH APPROACH

• Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east side of the new mainline US 119

JOHNS CREEK APPROACH

• Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach

V. EVALUATION PHASE

V.(a) ALTERNATIVES

ALTERNATIVES

The following alternatives were formulated during the "eliminate and combine" portion of the Evaluation Phase.

A. EXCAVATION

Value Engineering Alternative No. 1-Reduce the median width from a 12 m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach

Value Engineering Alternative No. 2-Revise the alignment between station 505 + 800 and station 507 + 300 to turn easterly along the hollow then back to the proposed alignment

Value Engineering Alternative No. 3-Flatten the side slopes in long fill sections (504 + 800 to 506 + 200 and 508 + 500 to 509 + 100)

B. STRUCTURES

Value Engineering Alternative No. 1-Reduce the shoulder width of the bridge typical section to 3.0m instead of the proposed 3.6m

C. US 119 AT BURNING FORK APPROACH

Value Engineering Alternative No. 1-Revise the north side of the intersection eliminating Ramp D to reflect a half diamond type interchange with the same long radius currently proposed for the SB to WB movement for the heavy trucks and also retaining the relocated US 119 configuration currently proposed for the south side of the intersection

D. RACCOON CREEK APPROACH

Value Engineering Alternative No. 1-Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp A and left of mainline station 502 + 900

E. WINN BRANCH APPROACH

Value Engineering Alternative No. 1-Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east of the new mainline US 119

F. JOHNS CREEK APPROACH

Value Engineering Alternative No. 1-Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach

V.(b) ADVANTAGES AND DISADVANTAGES

EVALUATION

The following Advantages and Disadvantages were developed for the Value Engineering Alternatives previously generated during the speculation phase. It also includes the Advantages and Disadvantages for the As Proposed.

A. EXCAVATION

As Proposed Typical Section (12m depressed median)

Advantages

- provides area for snow removal
- simplifies drainage
- reduces runoff on fill sections due to the crowned roadway section
- provides area wide enough to provide a refuge for smaller turning vehicles
- allows for provision of deceleration and acceleration lanes along the median
- a larger volume of excavated material would be utilized in fill sections
- eliminates all obstacles (barrier wall, etc.) from the median
- does not require milling of curb lips and barrier wall in future resurfacing operations

Disadvantages

- increases the amount of excavation required in cut areas
- increases the R/W requirements
- increases the cost of maintenance

Conclusion

Carry Forward for Further Development

Value Engineering Alternative No. 1 - Revise the median width from a 12m depressed median to a 4.2 m paved median with barrier wall throughout except at the Winn Branch Approach.

Advantages

- reduces the amount of excavation required
- reduces the R/W requirements
- reduces the amount of maintenance required to maintain the median
- still provides enough area for a left turn storage lane
- reduces the potential for head-on collisions due to the addition of the median barrier wall
- would reduce the bridge deck width by 1' 3 1/2"

Disadvantages

- does not provide enough width for refuge of smaller turning vehicles across the median
- barrier wall is considered a obstacle to vehicles
- eliminates some of the area that could be used for storage of snow
- complicates the drainage of the project due to the addition of median drainage boxes and loss of storage area
- requires the draining of pavement runoff across 2 lanes of traffic

Conclusion

Carry Forward for Further Evaluation

Value Engineering Alternative No. 2 - Revise the alignment between station 505 + 800 and station 507 + 300 to turn easterly along the hollow then back to the proposed alignment

Advantages

- will reduce the amount of excavation required
- may reduce the amount of R/W required
- may avoid the gas well at station 506 + 940
- allows a flatter profile grade along the mainline

Disadvantages

- will slightly increase the length of the roadway
- adds additional curves (2) to the alignment
- increases the potential impacts to the designated mine area
- eliminates a potential waste site

Conclusion

Carry Forward for Further Evaluation

Value Engineering Alternative No. 3 - Flatten the side slopes in long fill sections (stations 504+800 to 506+200 and 508 + 500 to 509 +100)

Advantages

- reduces the amount of excavated waste
- reduces the area needed for waste disposal
- more conducive to future development
- reduces the amount of guardrail required
- reduces the potential for fill slides

Disadvantages

- may increase the demand for additional access to the mainline roadway
- will increase the amount of drainage structures required
- will increase the amount of backslope that will have to maintained (mowing, etc.)

Conclusion

Carry Forward for Further Evaluation

B. STRUCTURES

Value Engineering Alternative No. 1 - Reduce the shoulder width of the bridge typical section to 3.0m instead of the proposed 3.6m

Advantages

- reduces the width of the bridges by 0.6m (2') each
- discourages the use of the outside shoulder as a traffic lane
- conforms to Kentucky Bridge Standards

Disadvantages

NONE

Conclusion

Carry Forward for Further Evaluation

C. US 119 AT BURNING FORK APPROACH

As Proposed Approach

Advantages

provides high operating speeds on Ramps C and D

Disadvantages

- requires a larger amount of excavation to construct
- requires a larger amount of pavement to construct
- requires additional R/W to construct
- requires an increased amount of drainage to construct
- design is more complex normally required for this type intersection

Conclusion

Carry Forward for Further Evaluation

Value Engineering Alternative No. 1 - Revise the north side of the intersection eliminating Ramp D to reflect a half diamond type interchange with the same long radius currently proposed for the SB to WB movement for the heavy trucks and also retaining the relocated US 119 configuration currently proposed for the south side of the intersection

Advantages

- reduces the amount of excavation required to construct
- reduces the amount of pavement required to construct
- reduces the amount of R/W required to construct
- reduces the amount of drainage required to construct
- design is similar to that normally used for a tight diamond intersection

Disadvantages

- will require a longer acceleration lane
- will reduce the operating speed of the interchange when compared to the As Proposed design

Conclusion

Carry Forward for Further Evaluation

D. RACCOON CREEK APPROACH

Value Engineering Alternative No. 1 - Reduce the number of graves to be removed by using a combination of slope reinforcement and retaining walls on Ramp A and left of mainline station 502 + 900

Advantages

- reduces the cost of grave relocation
- may help with public relations by reducing the social impacts of this project
- reduces the potential for project delay due to difficulties with grave relocation

Disadvantages

- increase the amount of waste material that will have to be disposed of elsewhere
- adds an additional cost for slope reinforcement and retaining wall

Conclusion

Carry Forward for Further Evaluation

E. WINN BRANCH APPROACH

Value Engineering Alternative No. 1 - Construct a wagon box to maintain access to Winn Branch Drive and eliminate the proposed approach on the east of the new mainline US 119

Advantages

- eliminates the only proposed at-grade crossing involving left turns in this project
- may decrease the amount of waste material
- retains the same access currently available to all the residents of Winn Branch Road

Disadvantages

- does not provide direct access to mainline US 119
- may increase the cost of construction, including drainage

Conclusion

Carry Forward for further Evaluation

F. JOHNS CREEK APPROACH

Value Engineering Alternative No. 1 - Use the mainline structures to provide for the separation of traffic between KY 194 and relocated US 119 and eliminate the proposed overpass on the new approach

Advantages

- eliminates the proposed overpass structure on the Johns Creek Approach
- may reduce the R/W requirements

Disadvantages

• may increase the length of the mainline structures

Conclusion

Drop From Further Consideration

VI. DEVELOPMENT PHASE

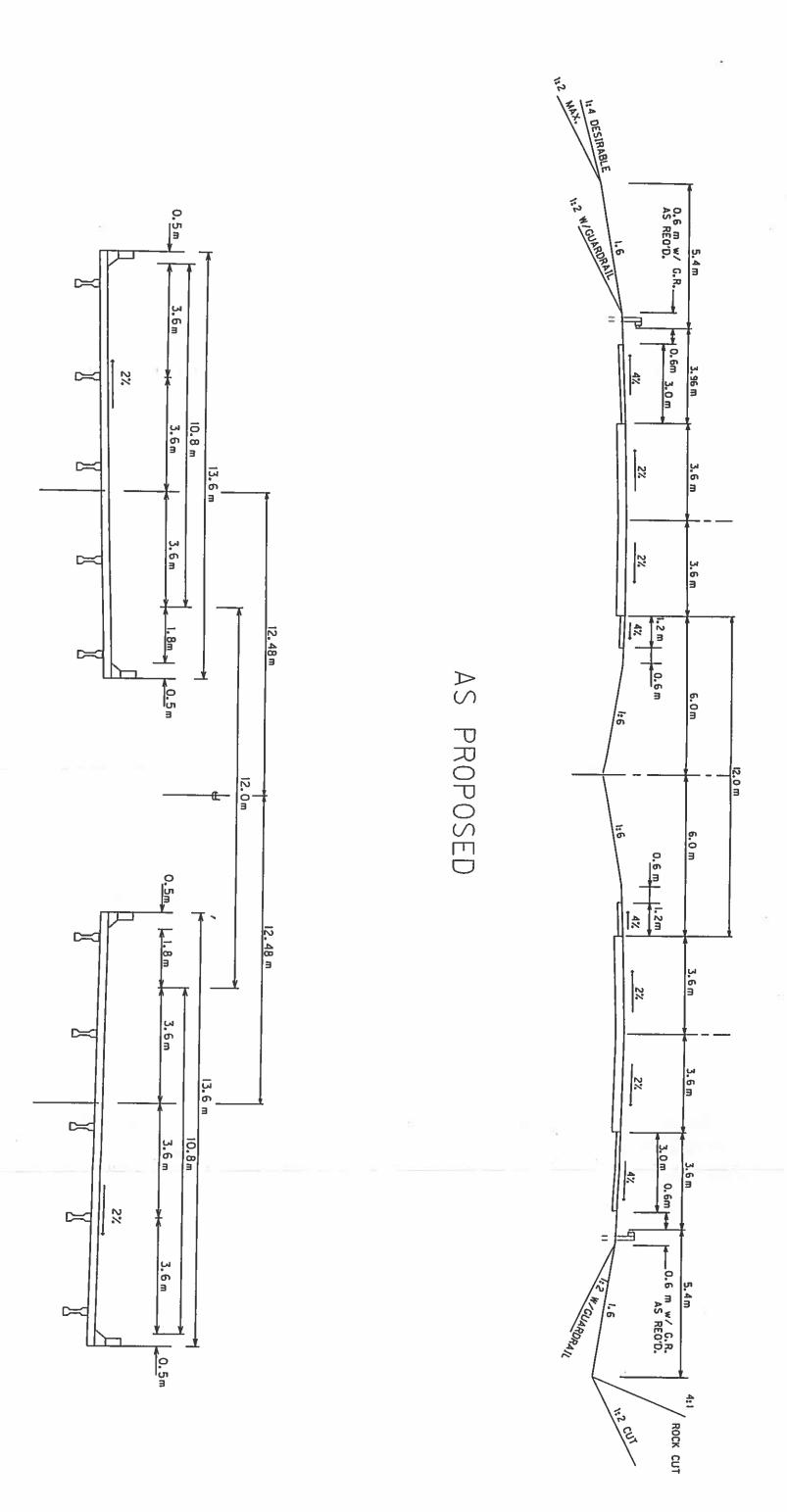
VI.(a) EXCAVATION

VI.(a)(1) AS PROPOSED

MEDIAN WIDTH

"AS PROPOSED" 12.0 m

The as proposed typical section incorporates four lanes at 3.6m, two median shoulders of 1.2m paved and .6m unpaved, two exterior shoulders at 3.0m paved and .6m unpaved. The median is a 12m depressed median. This section provides for drainage of both roadways and provides for snow storage, left turns and storage lanes.



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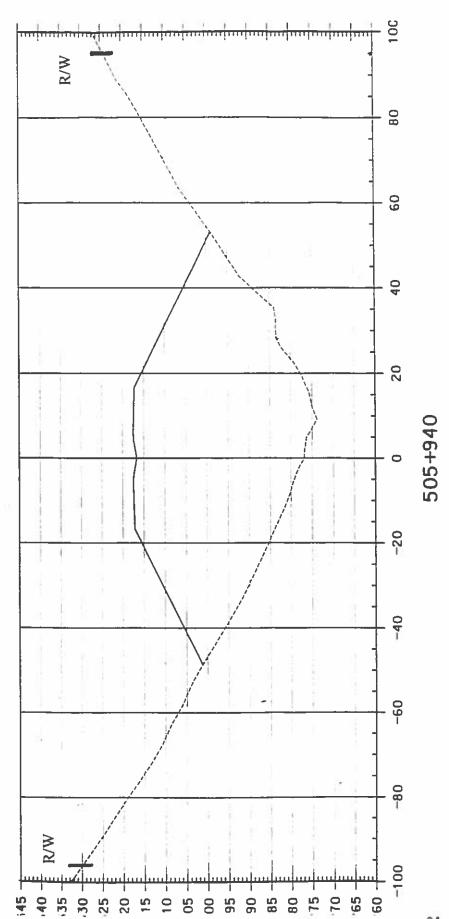
AS PROPOSED

AS PROPOSED

FILL SLOPES

"AS PROPOSED"

The proposed alignment provides for a typical section with a maximum slope of 1:2 in fill sections. Guardrail are utilized in areas steeper than 1:4 slopes.



ALIGNMENT

"AS PROPOSED"

The alignment between station 505 + 913 and station 507 + 225 crosses near the top of the mountain. This requires high volume excavation and adds to the waste disposal on the project.

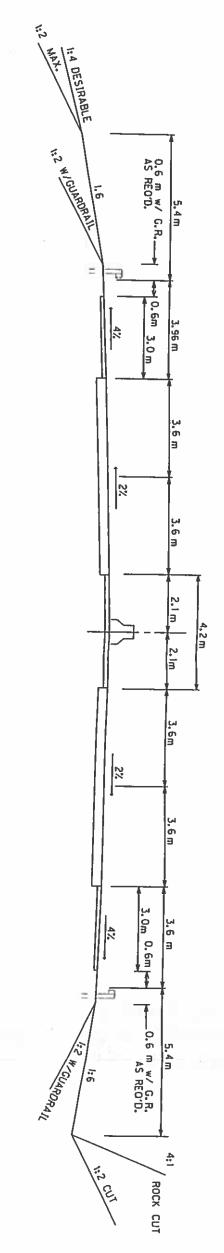
VI.(a)(2) V.E. ALTERNATIVES

MEDIAN WIDTH

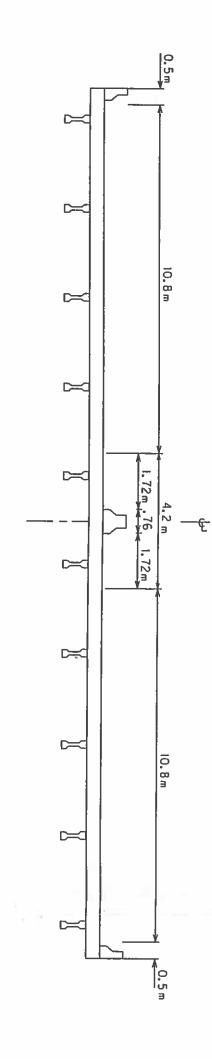
V.E. ALTERNATIVE NO. 1

The V.E. alternative typical section incorporates four lanes at 3.6m, two exterior shoulders at 3.0m paved and .6m unpaved. The median is 4.2m wide with a traffic barrier in the middle of the median. The barrier will be used throughout except where approach roads will have a left turn movement (Winn Branch only).

This typical provides for drainage, left turns and storage lanes.



VALUE ENGINEERING PROPOSAL



VALUE ENGINEERING PROPOSAL

COST COMPARISON

Revised Median Width (12m vs. 4.2m)

DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Pavement	\$110/m ²			11,041	\$ 1,214,510
Median Barrier Type 300C	\$140/m³			10,616	\$ 1,486,240
Crash Cushions	\$20,000/ea			4 ea.	\$ 80,000
Conc. Median Barrier Box Inlet	\$ 9,800/ea			43 ea.	\$ 421,400
Excavation Section 1	\$2.61/m ³	4,934,235	\$12,878,353	4,749,617	\$12,396,500
Section 2	\$2.61/m ³	8,194,128	\$21,386,674	7,722,398	\$20,155,458
Section 3	\$2.61/m ³	6,950,220	\$18,140,074	6,673,785	\$17,418,579
Section 4	\$2.61/m ³	4,468,968	\$11,664,006	4,210,184	\$10,988,580
Subtotal		24,547,551	\$64,069,107	23,355,984	\$64,161,267
Bridge Conc.	\$3.50/CY			148 C.Y.	\$ -51,800
Bridge Rebars	\$.55/LB			35,900 LB	\$ -19,745
TOTAL			\$64,069,107		\$64,089,722

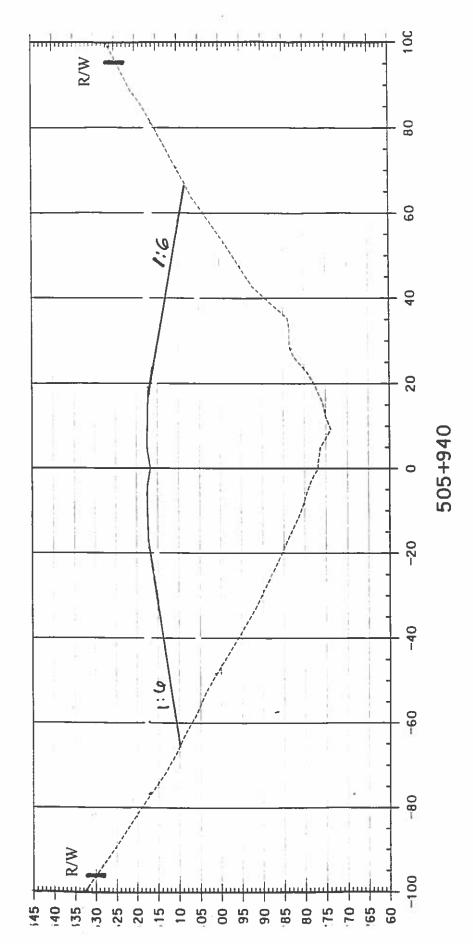
Possible Additional Cost \$ 518,985

FILL SLOPES

V.E. ALTERNATIVE NO. 2

The V.E. team recommends a typical section to provide modified slopes that will allow for the utilization of additional excavated material from station 504 + 800 to station 505 + 260, station

505 + 460 to station 506 + 200 and station 508 + 640 to station 509 + 060. This will reduce the amount of waste, the waste area required, and guardrail necessary.



helt End suardrail eliminated by waste usage or flattening slopes

Etat flatener 5 cpc 504+880 340 m 505+220 505+220 505+240 700 m 506+160 700 m 508+640 380 m 509+020

A600

Right side quardrail eveninated

Start flattened slope

End " 504+800 \(480 m \)

Start | 505+280 \(505+280 \)

Start | 506+200 \(506+200 \)

Start | 508+640 \(450 m \)

End " 509+060 \(450 m \)

Total Saved

3240m

STATION AREA	VOLUME	STATION	V AREA	VOLUME	STATION AREA	A VOLUME
504780	0	505440	0		508620	0
504800	18 180	505460	0 1954	19540	508640	1439 14390
	44 620	505480		40030	508660	1387 28260
504840	148 1920	505500	0 2160	42090	508680	1179 25660
504860 2	225 3730	505520	0 2132	42920	508700	1184 23630
504880 3	325 5500	505540	0 2362	44940	508720	1136 23200
504900 3	387 7120	505560	0 2168	45300	508740	1386 25220
504920 4	429 8160	505580	0 2458	46260	508760	1855 32410
504940 5	573 10020	205600	0 2299	47570	508780	2393 42480
504960 5	572 11450	505620	0 1933	42320	208800	1256 36490
504980 6	650 12220	505640	0 1812	37450	508820	1167 24230
505000 7	745 13950	505660	0 2055	38670	508840	1632 27990
505020 8	861 16060	505680	1504	35590	508860	2385 40170
505040 9	981 18420	505700	0 1174	26780	508880	2073 44580
505060 1145	45 21260	505720	0 1224	23980	508900	1385 34580
505080 1214	14 23590	505740	1816	30400	508920	640 20250
505100 1419	19 26330	505760	0 1845	36610	508940	439 10790
505120 1528	28 29470	505780	0 2099	39440	208960	320 7590
	32620	505800	0 1875	39740	508980	318 6380
505160 2002	02 37360	505820	0 1783		203000	348 6660
505180 2506	06 45080	505840	0 1640	34230	509020	276 6240
25	23 50290	505860		33440	509040	
	.,	505880		36040	209060	0 1050
505240 4:	435 4980	205900		33280		486060
505260 50	560 9950	505920	0 1482	29100		
505280	0 5600	505940	0 1036	25180		
SUM	421740 Cu.	J. M. 505960	0 961	19970		
		505980	914	18750		
		206000	1062	19760		
		506020	1166	22280		
		506040	1233	23990		
		506060	106	22960		
	45	506080	106			
		506100	146	25280		
		506120	0 1156	26220		
		506140	104	22040		
		506160		22000		
		506180	154	26920		
		506200	148			
		506220	0	14850		
	1					

COST COMPARISON

Flatten Fill Slopes

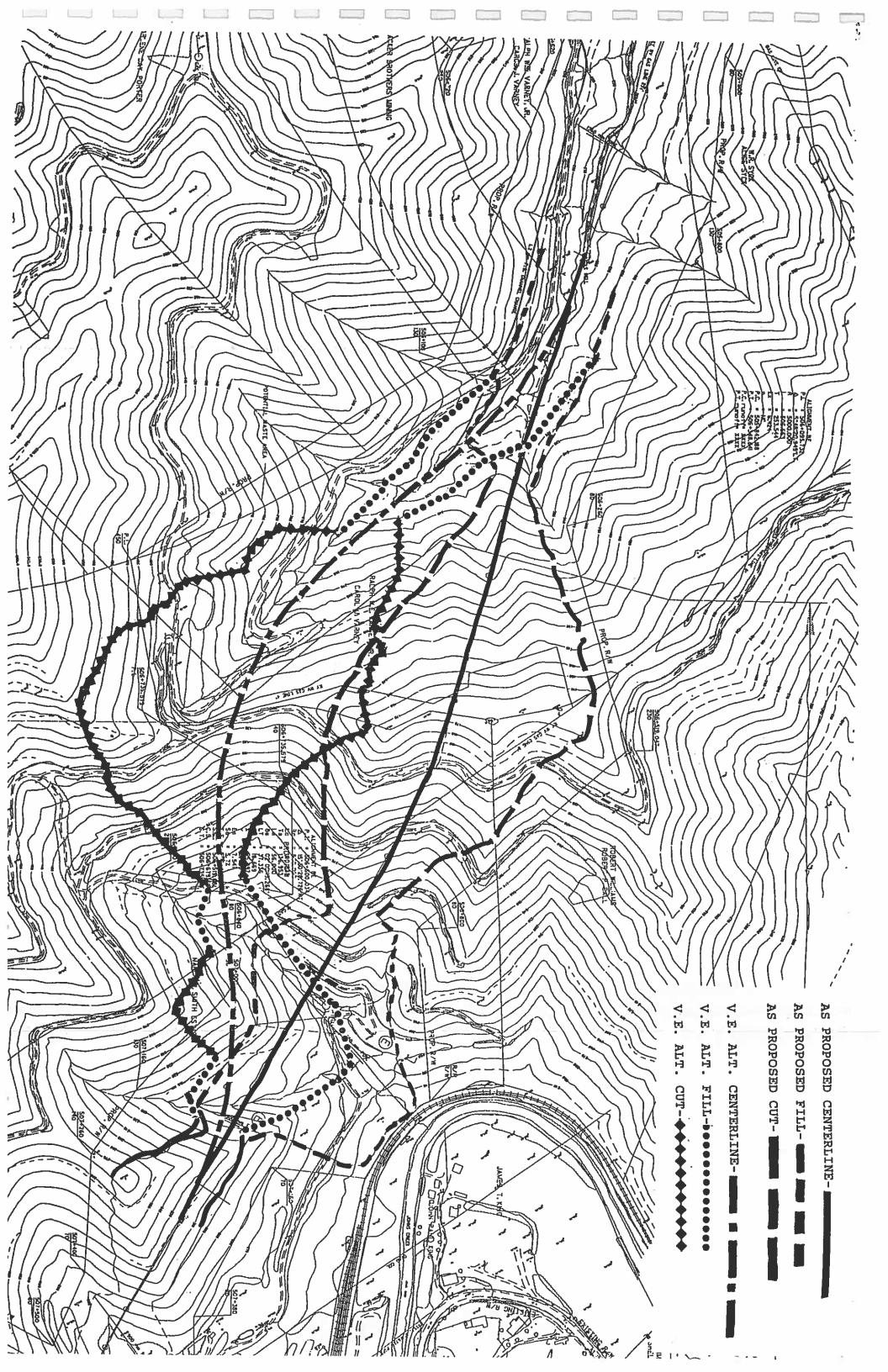
DESCRIPTION	UNIT	PROP'D	PROP'D	V.E.	V.E.
	COST	QTY.	COST	QTY.	COST
Guardrail	\$8.47/ft.	14,600 ft	\$123,662	3800 ft	\$32,186

Possible Savings \$ 91,476

ALIGNMENT

V.E. ALTERNATIVE NO. 3

The V.E. team recommends that the alignment be relocated through the saddle located southerly of the proposed alignment. This greatly reduces excavation and the volume of waste.



COST COMPARISON

V.E. Alternative No. 3 Alignment Revision

DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Roadway Pavement	\$6.81 Lin.M	1312 M	\$ 893,472	1376 M	\$ 937,056
Excavation	\$2.61 Cu.M	6,152,513	\$16,058,000	4,015,143	\$10,479,523
Drainage	\$224 Lin.M	1312 M	\$ 293,888	1376 M	\$ 308,224
TOTAL			\$17,245,360		\$11,724,803

Possible Savings \$ 5,520,554

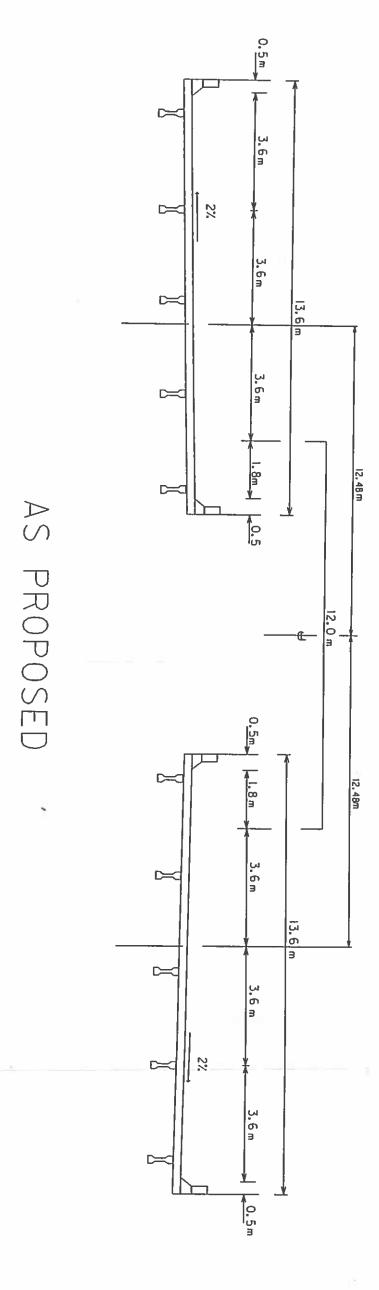
VI.(b) STRUCTURES

VI.(b)(1) AS PROPOSED

BRIDGE TYPICAL (EXTERIOR SHOULDER 3.6 VS. 3.0)

"AS PROPOSED"

The As Proposed Typical for the bridges incorporates 3.6m exterior shoulders, two 3.6m lanes, a 1.8m interior shoulder and two barriers at .5m each. This typical is for each bridge. The 3.6m exterior shoulder could encourage people to use this as a travel lane. (See AASHTO Geometric Design Chap. IV, Pg. 338).



VI.(b)(2) V.E. ALTERNATIVES

BRIDGE TYPICAL (EXTERIOR SHOULDER 3.6 VS. 3.0)

V.E. ALTERNATIVE NO. 1

The V.E. Alternative incorporates 3.0m exterior shoulders, two 3.6m lanes, a 1.8m interior shoulder and two barriers at .5m each. This typical reduces each bridge by .6m each. A 3.0m shoulder is adequate for emergency use and is consistent with the typical section for the adjacent Bent Mountain project and with the Basic Geometric Design Standards (Exhibit 66-03-06).

0.5 m 3.0m 2% 3.6m 13.0 m 3.6 m \sqsubset 1.8m | 0.5m 12.48 m 12.0 m 0.5m | 1.8m 12.48m 3, 6 m \bowtie 13.0 m \simeq 3.6 m 2% 3.0 m 0.5 m

VALUE ENGINEERING PROPOSAL

- 13. Space is provided for bus stops.
- 14. Improved lateral placement of vehicles and space for occasional encroachment of vehicles is provided.

For further information on other uses of shoulders, refer to NCHRP 254 (3).

Urban highways generally have curbs along the outer lanes. A stalled vehicle during peak hours disturbs traffic flow in all lanes in that direction when the outer lane serves through traffic. Where on-street parking is permitted, the parking lane provides some of the same services listed above for shoulders. Parking lanes are discussed further in the section "On-street Parking."

Width of Shoulders

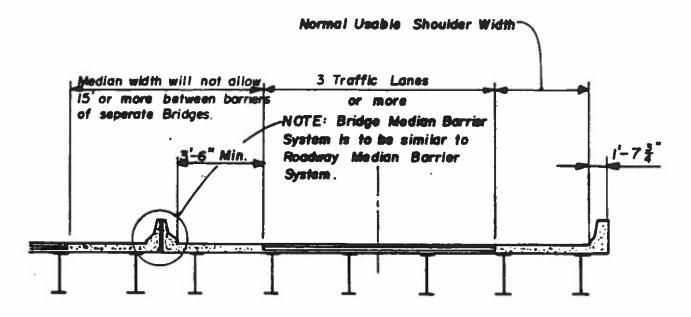
Desirably, a vehicle stopped on the shoulder should clear the pavement edge by at least 0.3 m, preferably by 0.6 m. This preference has led to the adoption of 3.0 m as the normal shoulder width that should be provided along high-type facilities. In difficult terrain and on low-volume highways, shoulders of this width may not be feasible. A minimum shoulder width of 0.6 m should be considered for the lowest-type highway, and a 1.8 to 2.4 m width would be preferable. Heavily traveled and high-speed highways and those carrying large numbers of trucks should have usable shoulders at least 3.0 m and preferably 3.6 m wide; however, widths greater than 3.0 m may encourage unauthorized use as a travel lane. Where bicyclists are to be accommodated, a minimum shoulder width of 1.2 m should be utilized. Shoulder widths for specific classes of highways are enumerated as parts of the total cross sections discussed in following chapters.

Where roadside barriers, walls, or other vertical elements are used, it is desirable to have a graded shoulder wide enough that these vertical elements can be offset a minimum of 0.6 m from the outer edge of the usable shoulder. It may be necessary to provide a graded shoulder wider than used elsewhere to provide lateral support for guardrail posts and/or clear space for lateral dynamic deflection required by the particular barrier in use. On low-volume roads, roadside barriers may be placed at the outer edge of the shoulder; however, a minimum of 1.2 m should be provided from the traveled way to the barrier.

Although it is desirable that a shoulder be wide enough for a vehicle to be driven completely off the traveled way, narrower shoulders are better than none at all. When a vehicle making an emergency stop can drive onto the shoulder to occupy only 0.3 to 1.2 m of a traveled way of adequate width, the remaining traveled way width can be used by passing vehicles. Partial shoulders are

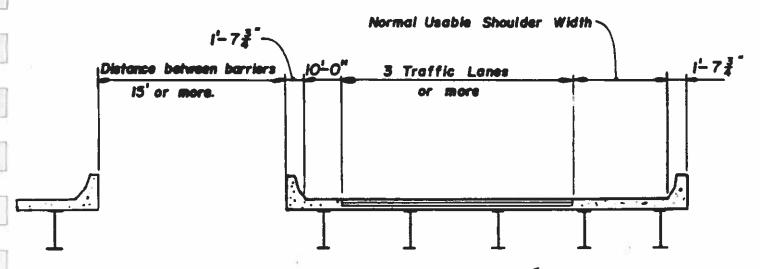
Commonwealth of Kentucky Transportation Cabinet Design Manual

BASIC GEOMETRIC DESIGN STANDARDS - FREEWAYS OVERPASS DESIGN



MULTI - LANE BRIDGES

OTHER THAN URBAN-MANDATORY
URBAN-DESIRABLE



DUAL MULTI - LANE BRIDGES

OTHER THAN URBAN-MANDATORY URBAN-DESIRABLE

NOTE:

On freeways exception is to be made for major long-span structures which warrant independent analyses for bridge-width determination.

STRUCTURES COST COMPARISON

V.E. Alternative No. 1 Revising Bridge Typical (Exterior Shoulder 3.6 vs. 3.0)

DESCRIPTION	UNIT	PROP'D	PROP'D	V.E.	V.E.
	COST	QTY.	COST	QTY.	COST
Burning Fork Conc.	\$3.50/CY			93.2 CY	\$ 32,620
Burning Fork Steel	\$.55/LB			22,600 LB	\$ 12,430
	-				
Racoon Creek Conc.	\$3.50/CY			160 CY	\$ 56,000
Racoon Creek Steel	\$.55/LB			38,880 LB	\$ 21,380
Johns Creek Conc.	\$3.50/CY			154 CY	\$ 53,900
Johns Creek Steel	\$.55/LB			37,400 LB	\$ 20,600
Bent Mountain Conc.	\$3.50/CY			44.6 CY	\$ 15,610
Bent Mountain Steel	\$.55/LB			10,800 LB	\$ 5,940
TOTAL					\$218,480

Possible Savings \$ 218,480

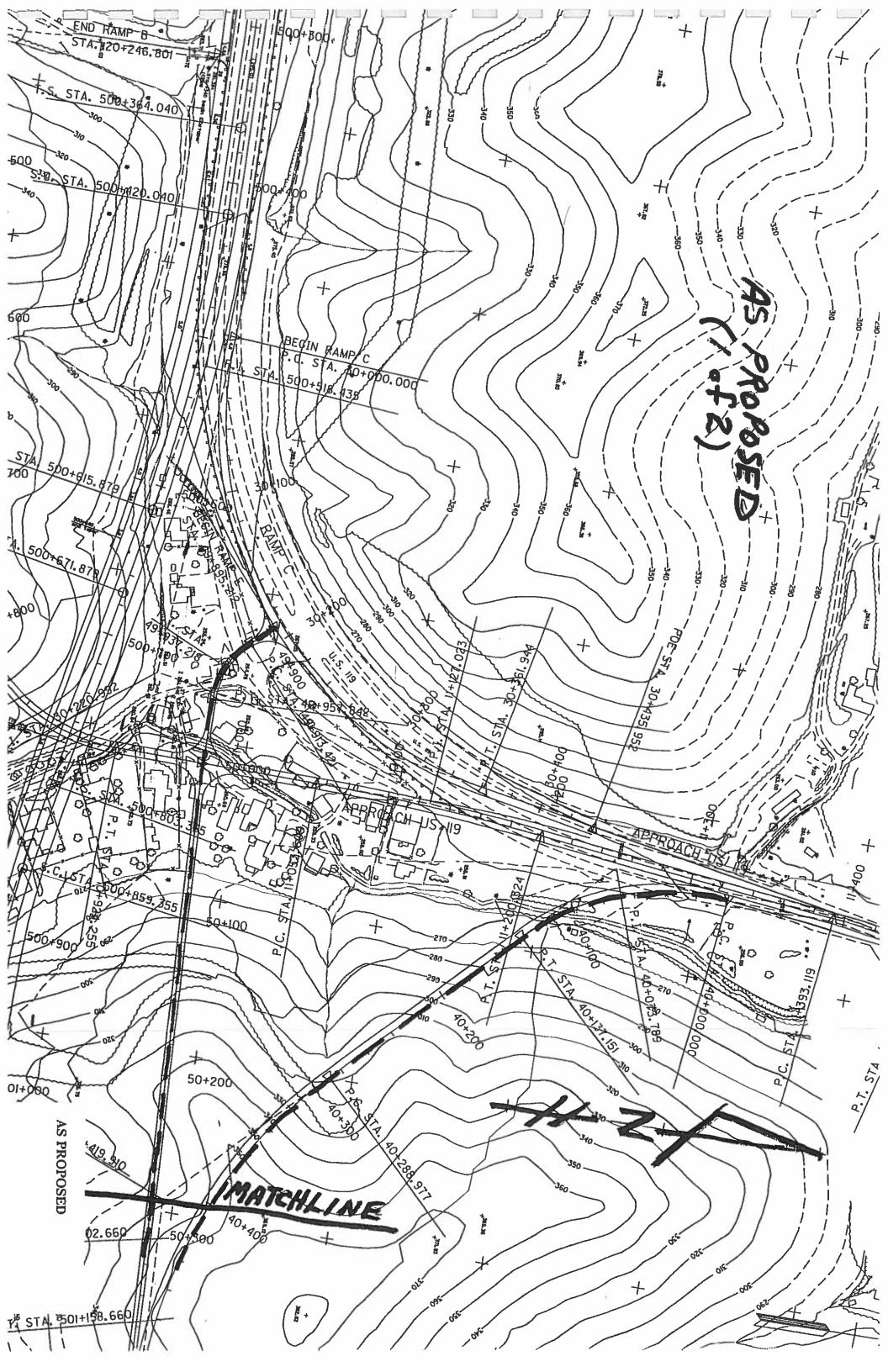
VI.(c) U.S. 119 AT BURNING FORK APPROACH

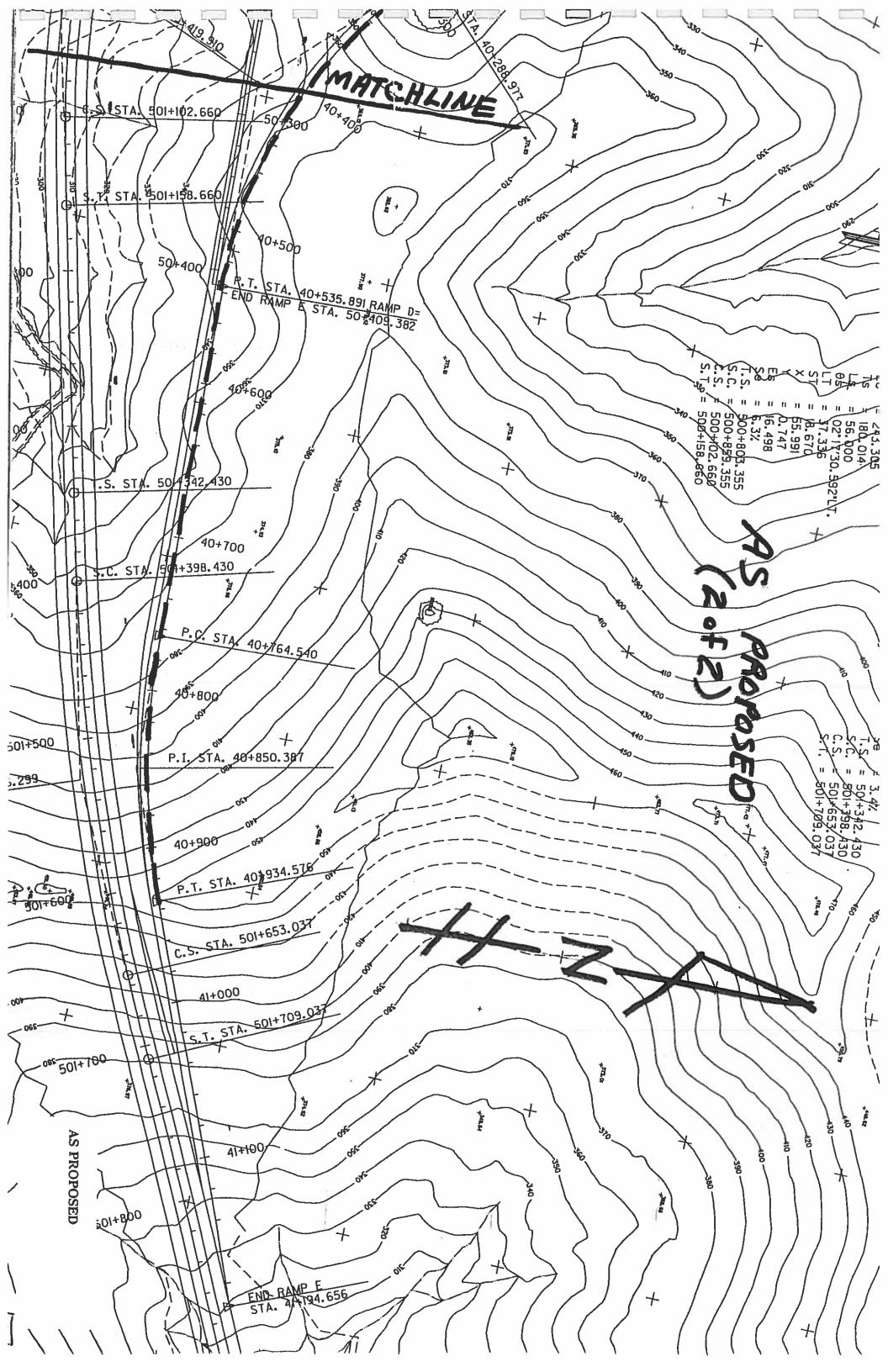
VI.(c)(1) AS PROPOSED

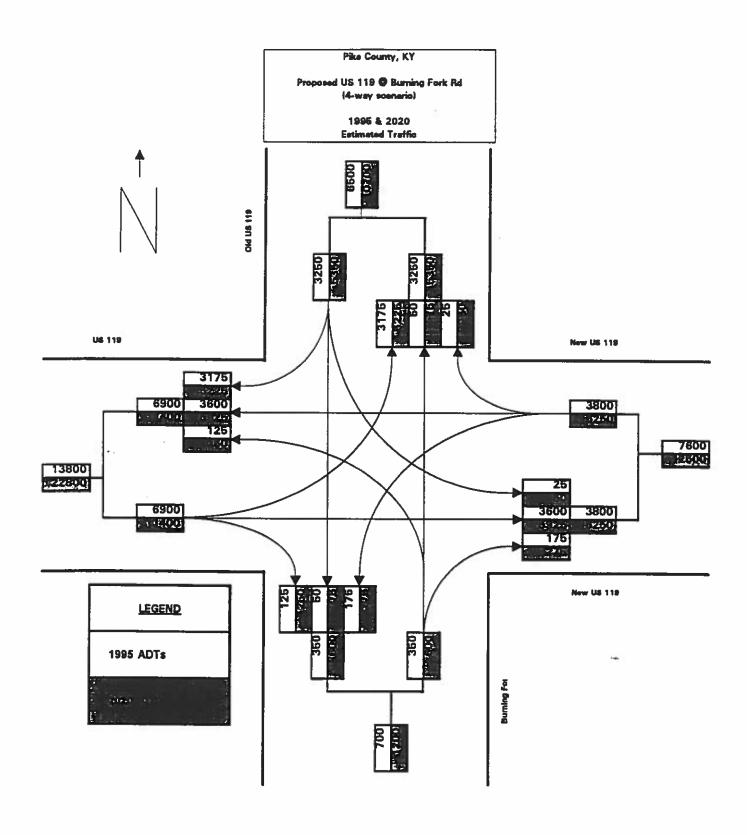
BURNING FORK

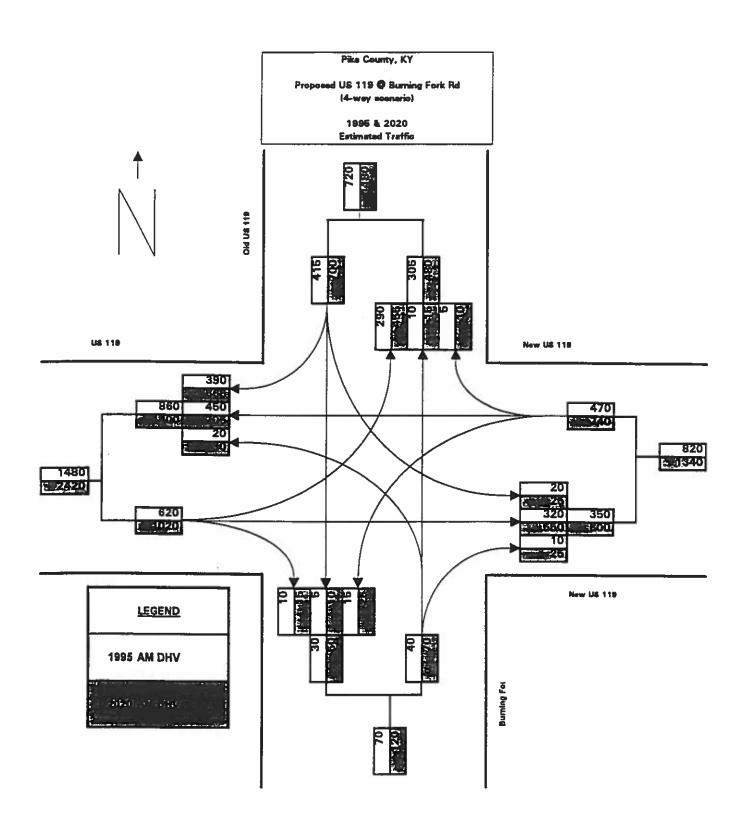
"AS PRUPUSED	PROPOSE	D"
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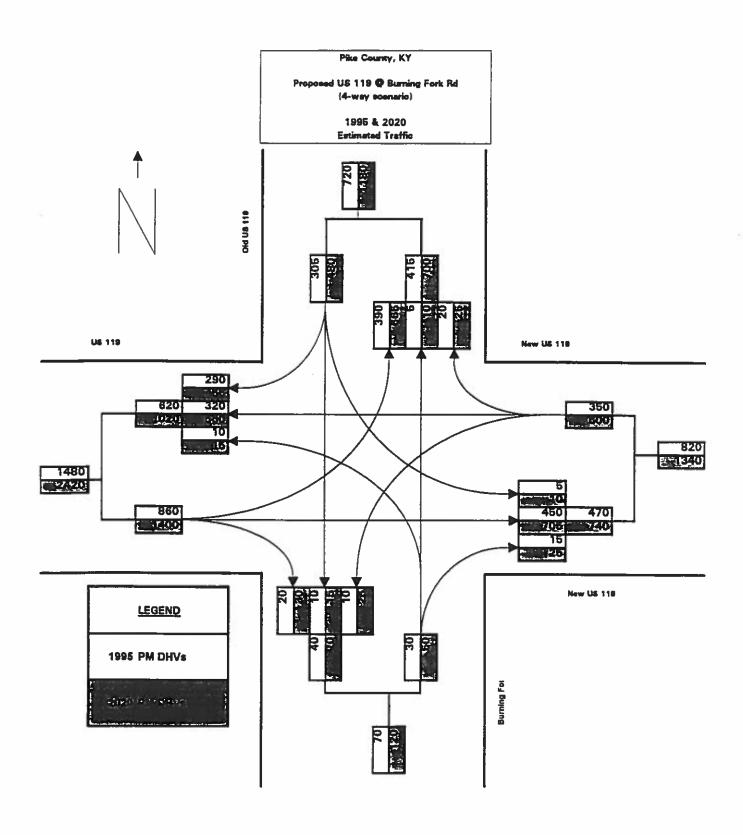
The proposed plan provides an off ramp (ramp D) from West to North for old U.S. 119. There is also a West to West ramp (ramp E) for Burning Fork Road South.









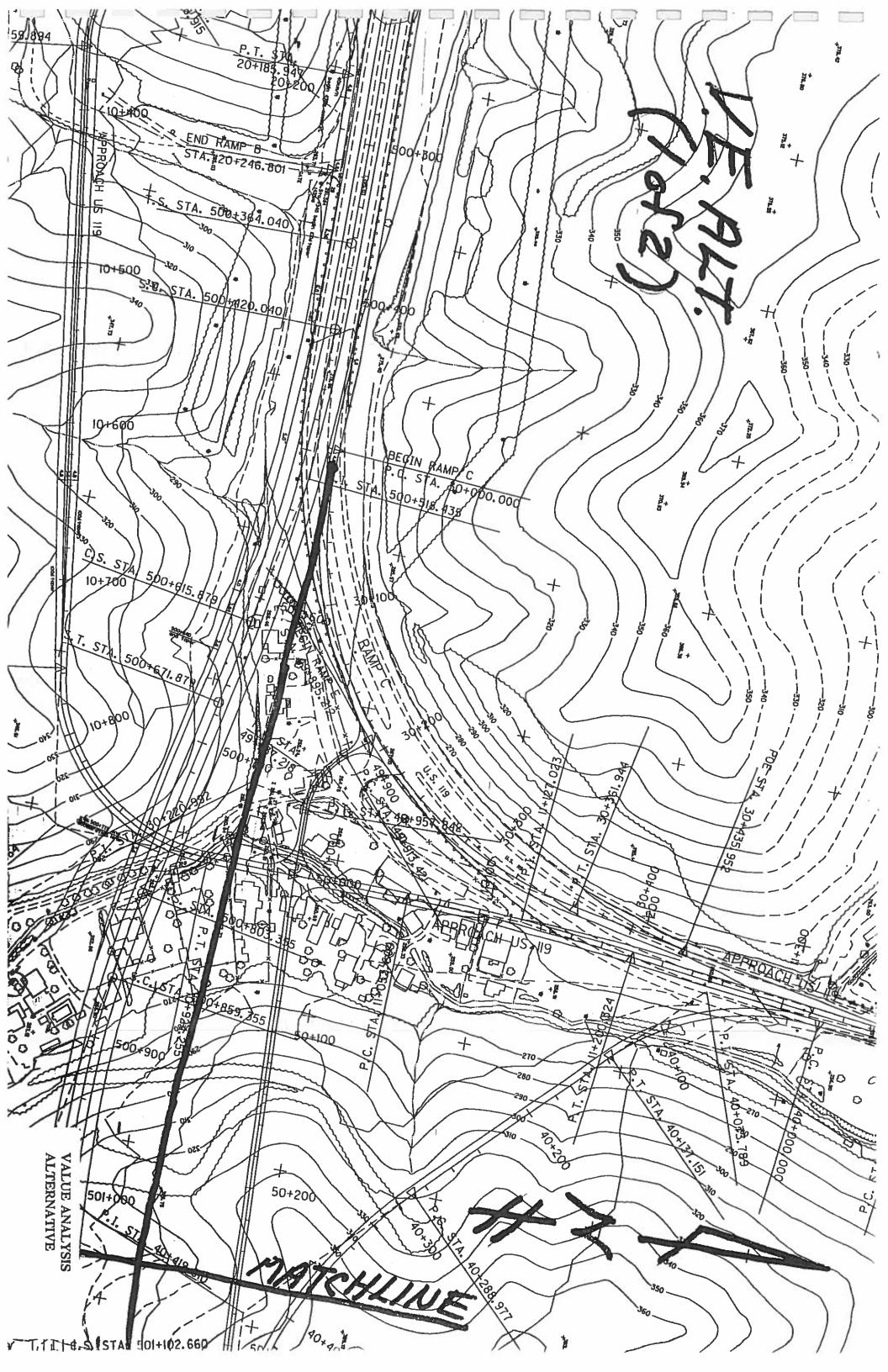


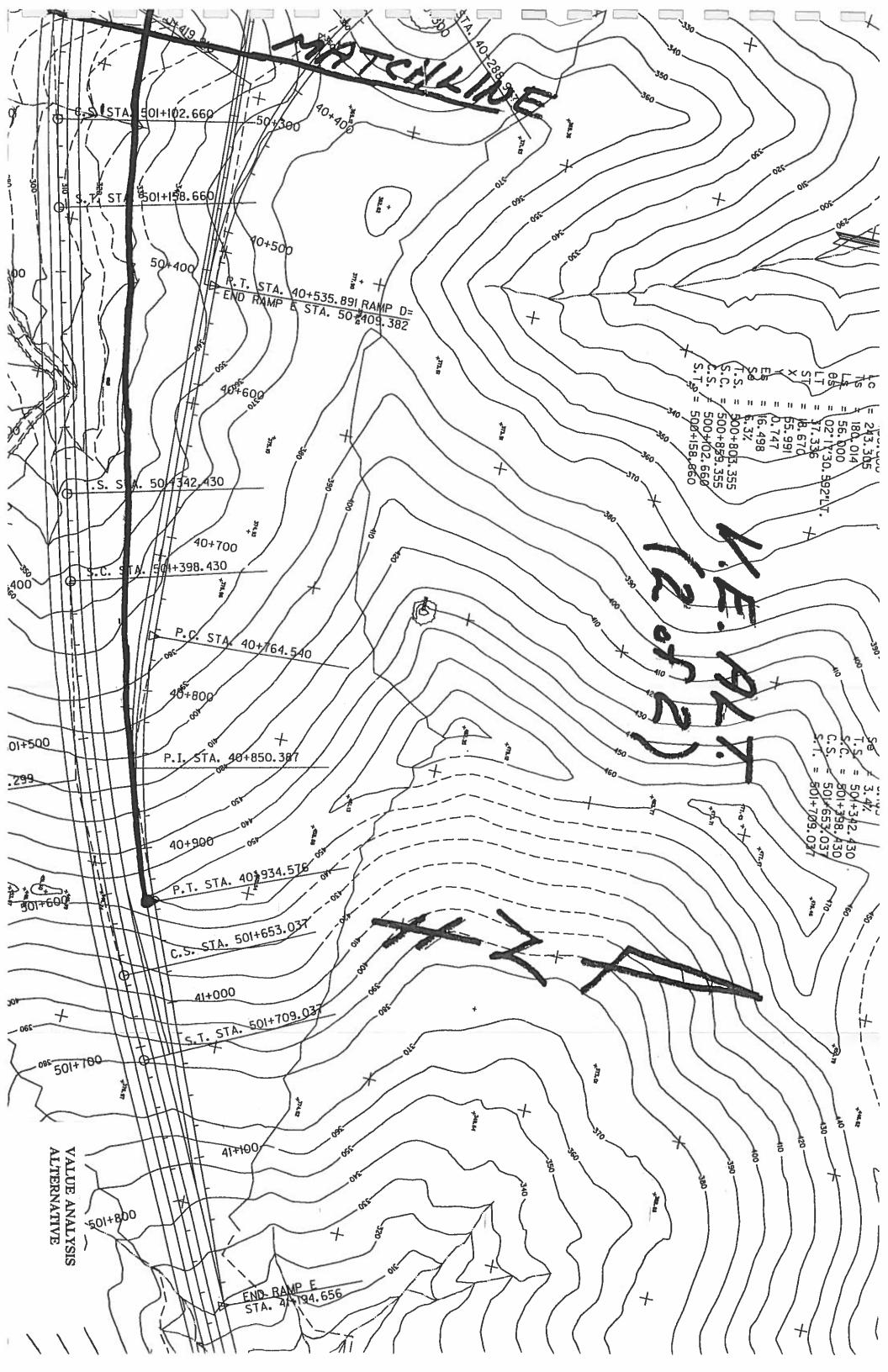
VI.(c)(2) V.E. ALTERNATIVES

BURNING FORK

V.E. ALTERNATIVE NO. 1

The V.E. team recommends that Ramp D be eliminated from station 40 + 000 to 40 + 535. Ramp E will be moved southerly toward the mainline and will intersect Ramp C at a more westerly location and closely (southerly) toward the mainline. Access north and south to old U.S. 119 (Burning Fork Road) is provided for west bound traffic along Ramp E. This eliminates more than 400M of ramp through a cut, reduces right of way requirements and waste.





COST SAVINGS

V.E. Alternative No. 1 Burning Fork

DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Right of Way	\$30,985/Ac.			(RED.) 19.4 Ac	(SAVINGS) \$ 601,103
Pavement	\$110/m ²			2,894m²	\$ 318,368
Excavation	\$2.61/m ³			1,367,550	\$3,569,306
				-	
TOTAL					\$4,488,777

Possible Savings \$4,488,777

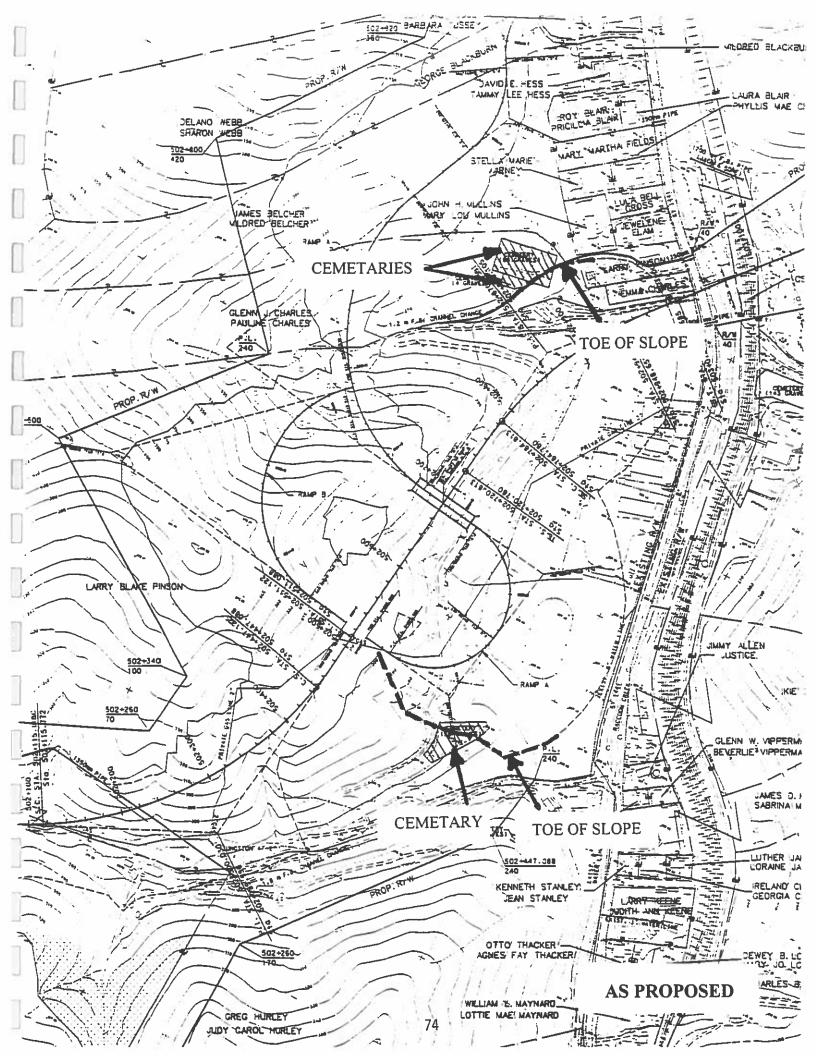
VI.(d) RACCOON CREEK APPROACH

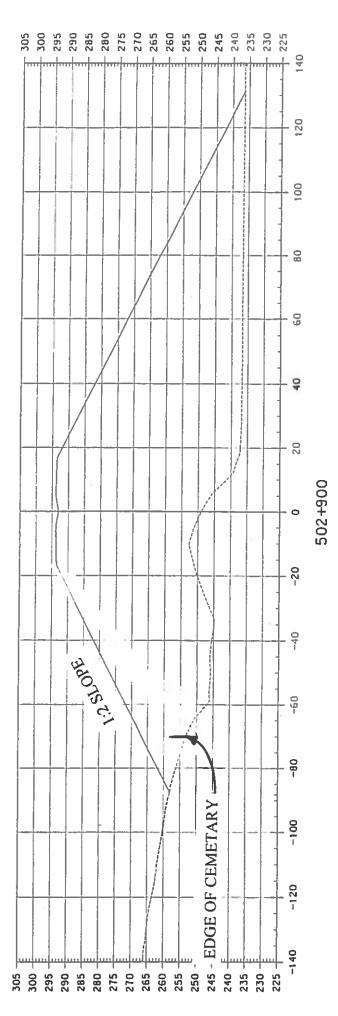
VI.(d)(1) AS PROPOSED

GRAVE REMOVAL

"AS PROPOSED"

The construction of fills for the proposed alignment and Ramp A at Raccoon Creek will force the relocation of three cemeteries. The cemetery right of station 502 + 500 contains 56 graves. The two cemeteries left of station 502 + 900 contain 4 and 68 graves respectively.





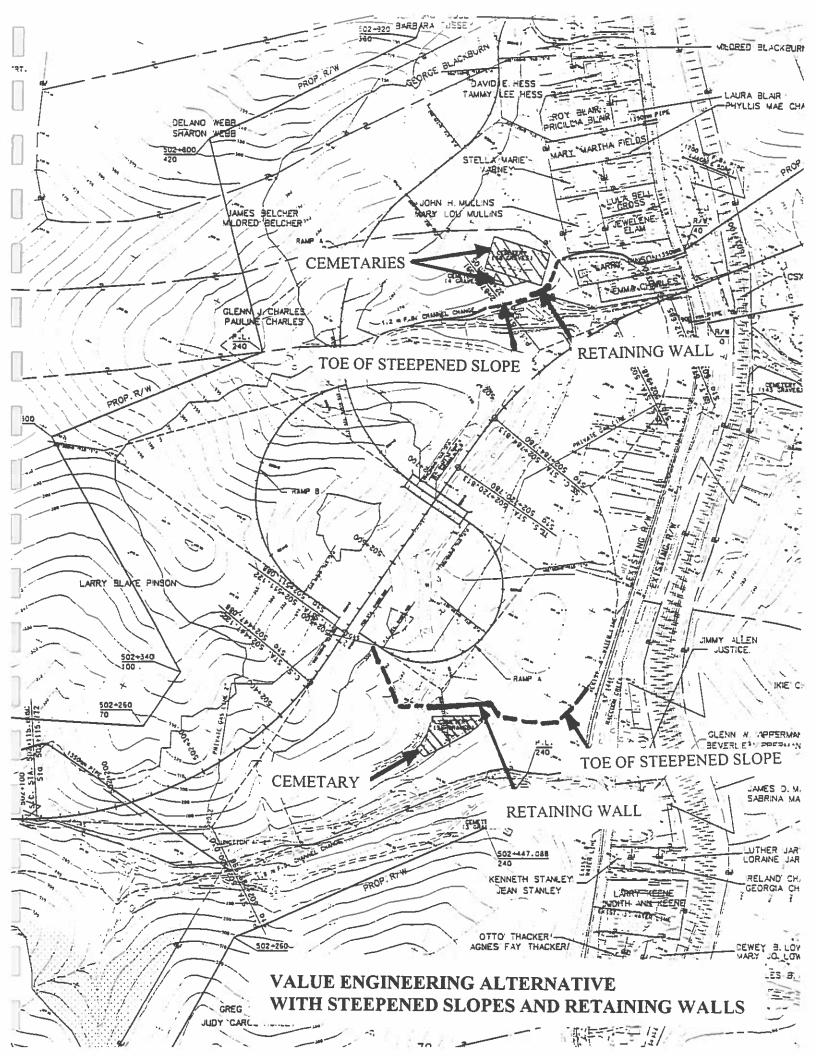
AS PROPOSED

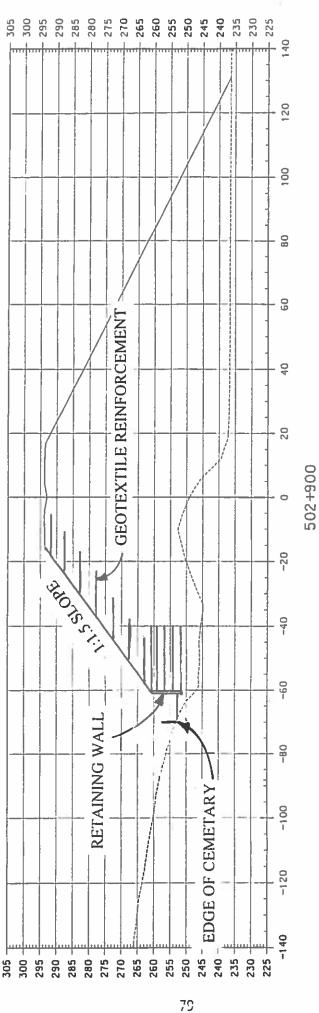
VI.(d)(2) V.E. ALTERNATIVES

GRAVE REMOVAL

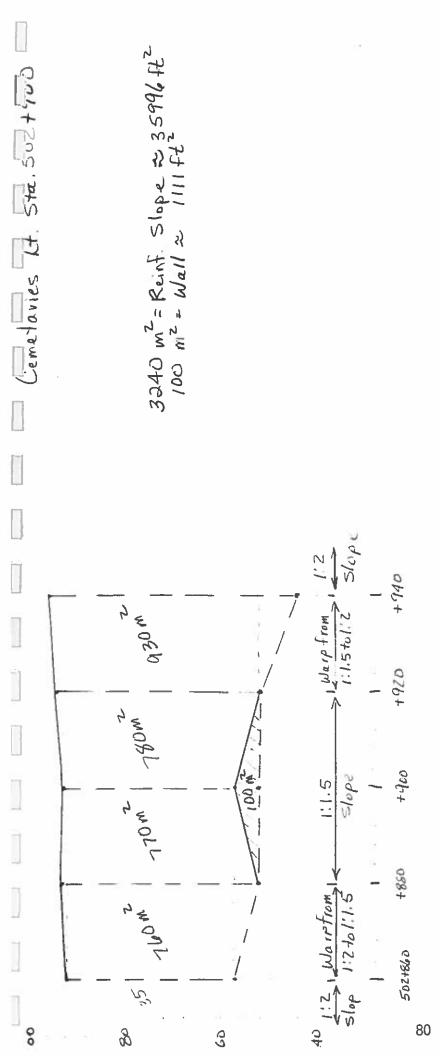
V.E. ALTERNATIVE NO. 1 - Use Retaining Walls & Steepened Slopes

This alternative uses MSE walls and 1:1.5 slopes reinforced with geotextile to reduce the footprint of the fill and avoid grave relocation.

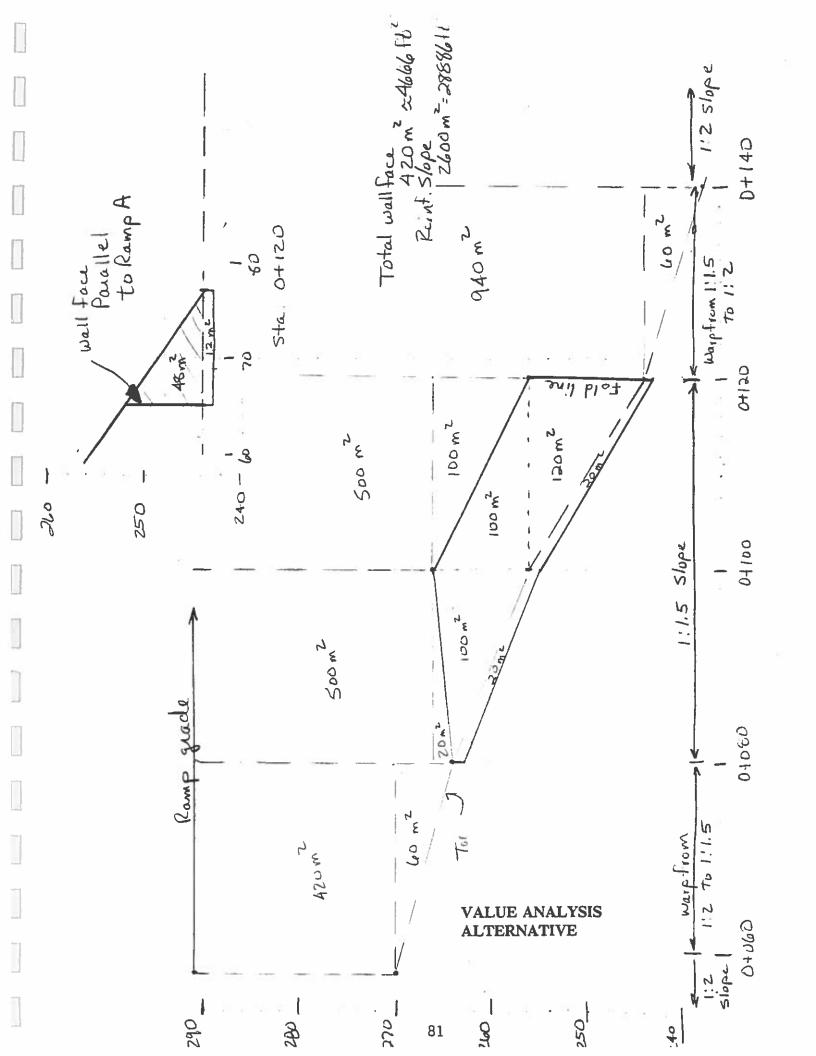




WITH STEEPENED SLOPES AND RETAINING WALLS VALUE ENGINEERING ALTERNATIVE



VALUE ANALYSIS ALTERNATIVE



COST COMPARISON

Cemetery Relocation vs. Ret. Walls & Steepened Slopes (1:11/2)

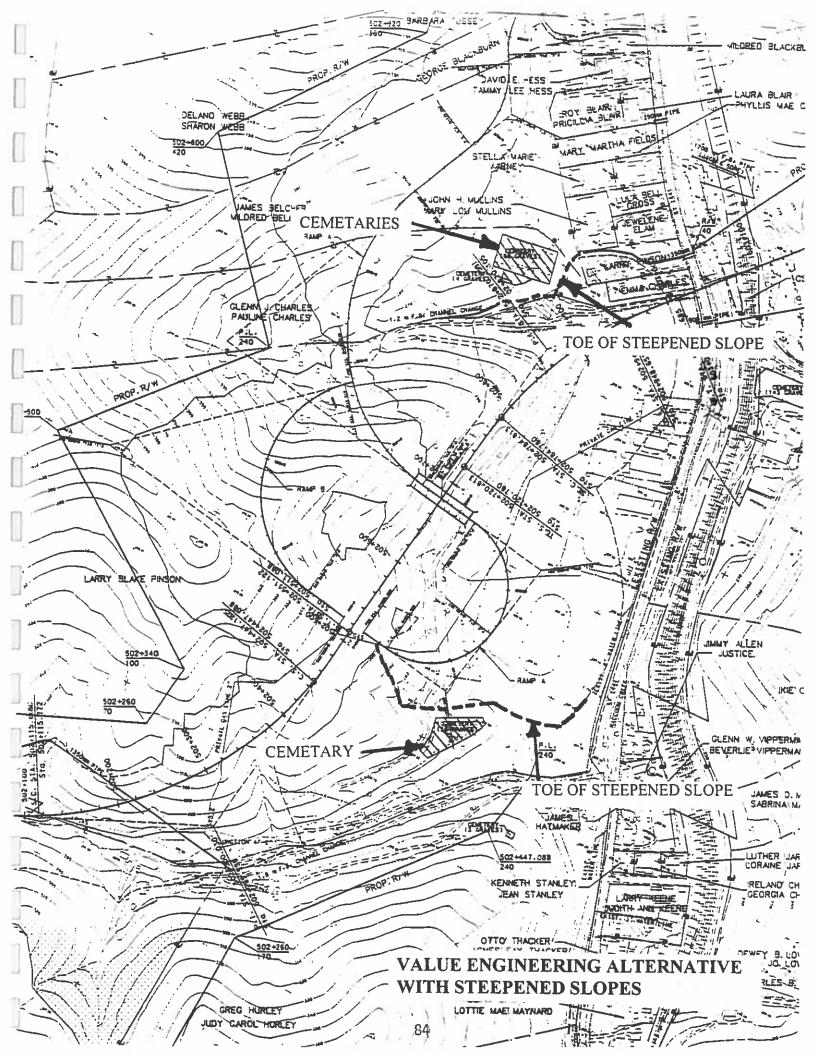
DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Grave Relocation	\$3,000/Grave	128	\$384,000		
					id.
Reinforced Steepened Slope	\$375/m ²			2600m ²	\$ 975,000
Reinforced Steepened Slope	\$375/m ²			3240m ²	\$1,218,000
					127
MSE Retaining Walls	\$430/m ²			420m ²	\$ 180,600
MSE Retaining Walls	\$430/m ²			100m ²	\$ 43,000
TOTAL			\$384,000		\$2,413,600
Conversion Factor 10.76 SF = 1m ²					

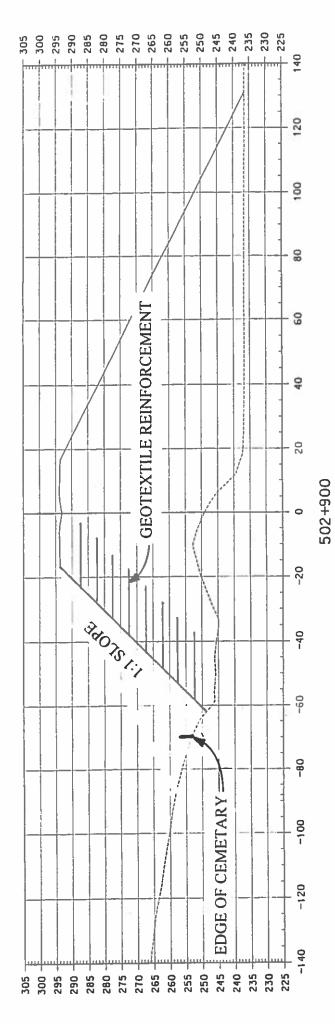
Possible Additional Cost \$ 2,029,600

GRAVE REMOVAL

V.E. ALTERNATIVE NO. 2 - Steepened Slopes

This alternative uses 1:1 slopes reinforced with geotextile to reduce the footprint of the fill and avoid grave relocation.





VALUE ENGINEERING ALTERNATIVE WITH STEEPENED SLOPES

COST COMPARISON

Cemetery Relocation vs. Steepened Slopes (1:1)

DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Grave Relocation	\$3,000/Grave	128	\$384,000		-
Reinforced Steepened Slope	\$460/m ²	12		3020m ²	\$1,389,200
Reinforced Steepened Slope	\$460/m ²			3340m ²	\$1,536,400
TOTAL			\$384,000		\$2,925,600

Possible Additional Cost \$ 2,541,600

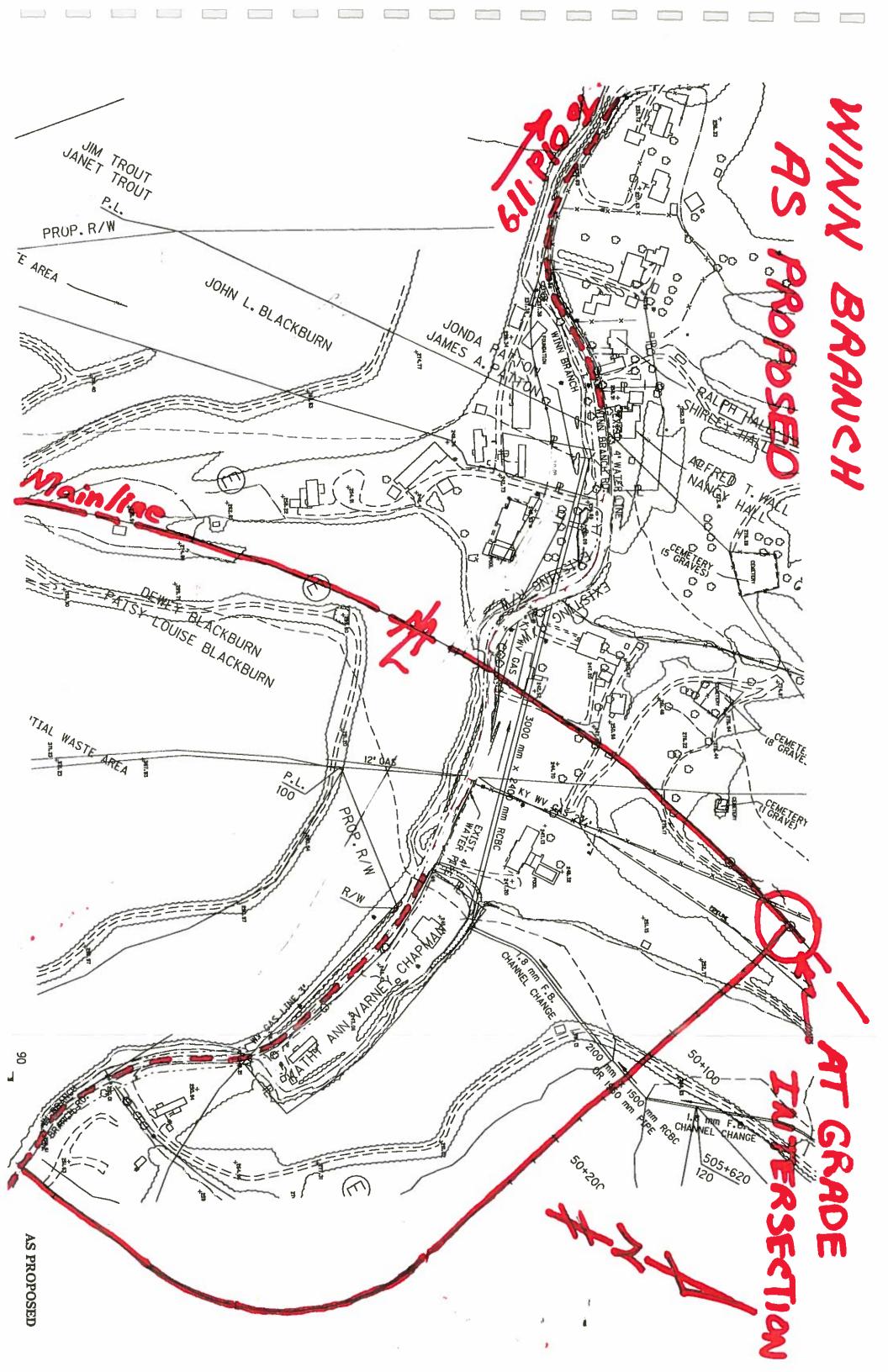
VI.(e) WINN BRANCH APPROACH

VI.(e)(1) AS PROPOSED

WINN BRANCH (505 + 300)

"AS PROPOSED"

The mainline alignment intersects Winn Branch Road at Mainline Station 505 + 300 (\pm). The as proposed solution is to cut off Winn Branch road on both side of the embankment. Those residences north of the mainline maintain their existing access to the North to old U.S. 119. They would have no direct access to neighbors south of the mainline. Residents south of the mainline would have access to the mainline only by way of a new access road that would intersect the mainline at station 505 + 575. The mainline intersection would be at grade and would allow south to west turns across the median. This would be contrary to a project design criteria that stated there should be no median crossings. This intersection would be the only exception on the entire project.



VI.(e)(2) V.E. ALTERNATIVES

WINN BRANCH

V.E. ALTERNATIVE NO. 1

The V.E. alternative provides no access to the mainline but does maintain existing access to old U.S. 119 for all residences on Winn Branch Road. The V.E. alternative eliminates the proposed access road and the at grade intersection on the mainline and utilizes a $8.5m \times 4.8m$ Wagon Box through the embankment at station 505 + 300. The primary advantages her would be:

- 1. Maintaining neighborhood integrity.
- 2. Equal access for all Winn Branch Rd. residences.
- 3. Elimination of at grade intersection and resultant median crossing.

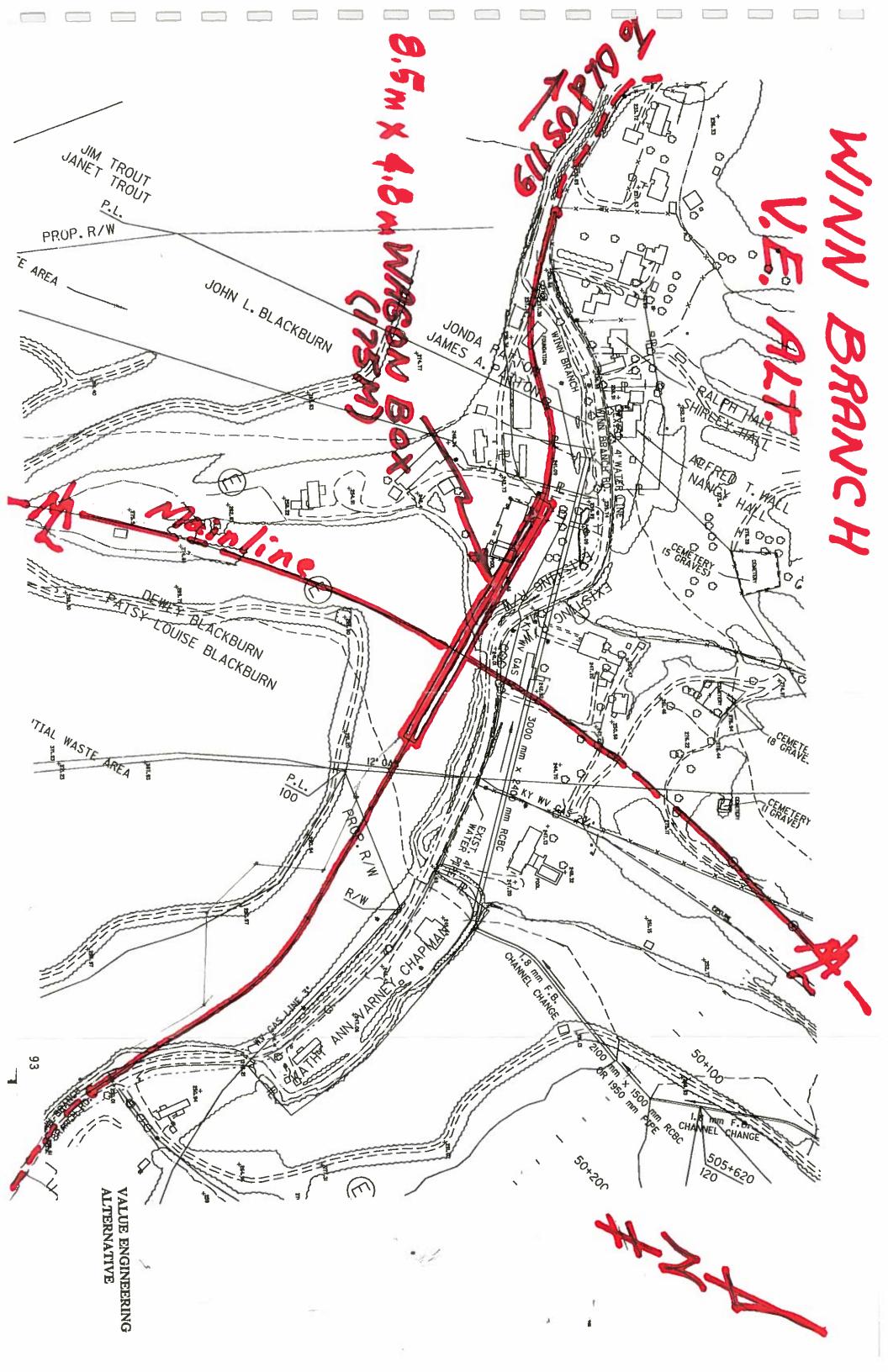
The primary disadvantages would be:

1. Increased cost.

As proposed = \$2.898 (2.124 + *775 R/W)

V.E. Alternative = \$4.714

- 2. No direct access to new facility.
- * $R/W = 25 \text{ Acres } @ $30,985/Acre}$



4.8 m Ugon Box · · ·

8 <u>B</u>

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VE ALT.

COST COMPARISON

Winn Branch

DESCRIPTION	UNIT COST	PROP'D QTY.	PROP'D COST	V.E. QTY.	V.E. COST
Access Road			\$2,123,375	0	0
Wagon Box		0	0		\$4,714,000
Right of Way	30,985 Ac	25	\$ 774,625	0	0
		·			
TOTAL			\$2,898,000		\$4,714,000

Possible Additional Cost \$ 1,816,000

VII. SUMMARY OF RECOMMENDATIONS

SUMMARY OF RECOMMENDATIONS

It is the recommendation of the Value Engineering Team that the following Value Engineering Alternatives be carried into the Project Development process for further development.

EXCAVATION

Recommendation No. 1

The Value Engineering Team recommends that Value Engineering Alternative No. 2 be implemented. This alternative is to flatten the fill slopes in areas with long fills to a 1:6 slope, reducing the amount of waste material.

If this recommendation can be implemented, there is a potential savings of \$91,476.

Recommendation No. 2

The Value Engineering Team recommends that Value Engineering Alternative No. 3 be implemented. This alternative is to revise the alignment between stations 505+800 and 507+300.

If this recommendation can be implemented, there is a potential savings of \$5,520,554.

STRUCTURES

Recommendation No. 3

The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to reduce the bridge shoulder widths to 3.0m (10 feet).

If this recommendation can be implemented, there is a potential savings of \$218,480.

US 119 AT BURNING FORK

Recommendation No. 4

The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to revise the design of the US 119 at Burning Fork Road interchange.

If this recommendation can be implemented, there is a potential savings of \$4,488,777.

WINN BRANCH APPROACH

Recommendation No. 5

The Value Engineering Team recommends that Value Engineering Alternative No. 1 be implemented. This alternative is to eliminate the proposed at-grade intersection and construct a wagon box along Winn Branch Road.

If this recommendation can be implemented, there is an additional cost of \$1,816,000.

If all these recommendations are implemented, there is a potential total savings of approximately \$8,503,287.

US 119/ZEBULON TO BENT MOUNTAIN V.E. STUDY PRESENTATION January 14, 1997

NAME	AFFILIATION	PHONE
Jack Trickey	Ventry Engineering	904/627-3900
Ron Whichel	Ventry Engineering	904/627-3900
Dallas Gray	Ventry Engineering	904/627-3900
Daryl Greer	KTC Co. Hwy. Design	502/564-3280
Ken Sperry	KTC Co. Design	502/564-3280
Don Keenan	Ventry Engineering	904/627-3900
David Lindeman	Palmer Engineering	606/744-1218
Randy Stephens	Palmer Engineering	606/744-1218
John Sacksteder	KTC - Design	502/564-3280
Bill Hornbeck	KTC - Bridge Design	502/564-4560
Joette Fields	KTC - Design	502-564-3280
Charles Briggs	Div. Operations	506/564-4556
Keith R. Damron	Dist. Design Engineer	606/433-7791

VIII. APPENDICES

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CONSTRUCTION COST ESTIMATE ALTERNATE B **CONSTRUCTION SECTION 1**

STA, 500+000 - 503+480 = (3480m) (3.48 km) -- (11417 FT.) (2.163 mi.)

LENGTH ≈ (3480m) (3,48 km)	(11417 F	T.) (2.163 mi;)	
DESCRIPTION	CHANTER	UNIT	COST	TOTAL
والمراجع والمناور والمراجع والم والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع والمراج	QUANTITY		\$2.81	TOTAL
EXCAVATION	5,080,749	CU. METER		\$13,260,75
SITUATION SIZE CROSS DRAINS	LS	LS	LS	\$181.99
MEDIAN CROSS DRAINS	LS	LS	LS	\$144,00
MEDIAN BOXS	50	EACH	\$2,300.00	\$115.00
PERFORATED PIPE UNDERDRAIN 4"	57,085	LIN. FT.	\$5.00	\$285,42
CHANNEL CHANGE	2.222	ÇU.YO.	\$2.01	\$
CHANNEL LINING CLASS 4	6,920	TÓN	\$3.58	\$24,77
LEARING AND GRUBBING		ACRES	\$1,200.00	Ş
SILT CHECKS	30	EA.	348.47	\$1,45
SUARORAIL	5,000	LIN,FT.	\$8,47	\$42,35
NO TREATMENTS	20	EA.	\$500.00	\$10,00
STAKING	2,163	MILE	\$45.000.00	\$97,33
W FENCE	22,400	UN.FT.	\$3.08	\$68,99
MAINTAIN AND CONTROL TRAFFIC	1	LUMP	\$150,000,00	\$150,00
VATER	2,000	MGAL	\$2.49	\$4,98
1 OGA	23,042	TON	\$12.00	\$276.50
P DRAINAGE BLANKET	28.699	TÓN	\$21.00	\$602,67
O' BASE	42,738	TON	\$25.37	\$1,084,26
.5" BITUMINOUS CONC. SURFACE	8.098	TON	\$24.93	\$201,88
TULL DEPTH DGA	14,258	TON	\$11.38	\$161.97
SITUMINOUS MATERIAL FOR TACK	77.2	TON	\$238.75	\$18,43
MULSIFIED ASPHALT RS-2	24.3	TÓN	\$291.18	\$7,07
STUMINOUS SEAL AGGREGATE	202	TÓN	\$25.87	35,24
SEED AND PROTECTION		SO.YO.	\$0.18	\$
SUB TOTAL				\$16,745,10
MOBILIZATION 3%				\$502.35
DEMOBILIZATION 1,5%				\$251,17
JATOT BUS				\$17,498,63
NGINEER. & CONTING. 20%				\$3,499,72
MAINLINE TOTAL		-	-	\$20,998,36

OPTIONS FOR ALT. B APPR. TO US 119 AT B		
APPROACH AT. STA. 500+340	\$7,746,766	
APPROACH RT. STA. 501+000	\$1,592,610	
APPROACH LT. STA. 500+340	\$4,605,028	
APPROACH LT. STAL 501+000	\$2,460,725	
TUAN LANES (2)	\$124,966	
BRIOGE	\$5.521.245	5.561.64
CULVERT	\$615,600	
		M # 545 47 5

		9.8	
OPTIONS FOR ALT. 8 -	SECTION 1	40°	40.00
APPROACH AT RACCOON	CREEK KY 1441	400	The state of the s
APPROACH LT. STA. 502+040	\$10,563,016	Т	
APPROACH RT. STA. 502+100	\$546,233		
APPROACH RT. STA. 502-040	\$14,662,468	T	
TURN LANES (2)	\$124,960	T	A
BRIDGE	\$9,865,733	$oldsymbol{oldsymbol{oldsymbol{\Box}}}$	7. 365.17

BURN 1 9, 201,030 - 7, 820,912

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PIKE COUNTY US 119

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GRAND TOTAL

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PIKE COUNTY US 119

CONSTRUCTION COST ESTIMATE ALTERNATE B CONSTRUCTION SECTION 2 STA, 503+480 - 506+320 LENGTH = {2840 m} (2.840 km) -- (9317 ft.) (1.76 mi.) UNIT DESCRIPTION QUANTITY UNIT COST TOTAL EXCAVATION 7,739,105 CU, METER \$2.61 \$20,199,064 SITUATION SIZE CROSS DRAINS LŞ. LS \$84,307 MEDIAN CROSS DRAINS LS \$101.049 LS LS MEDIAN BOXS 35 EACH \$2,300.00 \$80,500 PERFORATED PIPE UNDERDRAIN 4" 37,268 LIN. FT. \$5.00 \$186,340 CHANNEL CHANGE CU.YD. \$0 \$2,01 CHANNEL LINING CLASS 4 3,558 TON \$3.58 \$12,738 CLEARING AND GRUBBING ACRES \$1,200.00 \$0 SILT CHECKS 20 EA. \$48,47 \$969 \$27,104 GUARDRAIL 3,200 UN,FT, 58,47 END TREATMENTS ĘA, \$500.00 \$7,000 14 STAKING 1.76 MILE \$45,000.00 \$79,200 AW FENCE . 18,634 UN.FT. \$3.06 \$57,393 MAINTAIN AND CONTROL TRAFFIC 1 LUMP \$50,000,00 \$50,000 WATER 2,000 MGAL \$2.49 \$4,980 TÓN 4" DGA 18,871 \$226,452 \$12.00 4" DRAINAGE BLANKET 23,504 TON \$21.00 \$493,584 10' BASE 34,996 TON \$25.37 \$887,849 1.5' BITUMINOUS CONC. SURFACE TON 6,633 524.93 \$165,361 FULL DEPTH DOA 11677 \$132,651 \$11.36 BITUMINOUS MATERIAL FOR TACK TON 63 \$230.75 \$15,017 EMULSIFIED ASPHALT RS-2 TÓN 40 \$291.18 \$11,560 BITUMINOUS SEAL AGGREGATE 331 TON \$25.97 \$8.596 SEED AND PROTECTION SO.YD. \$n \$0.18 SUB TOTAL \$22,831,713 MOBILIZATION 3% \$684,951 DEMOBILIZATION 1.5% \$342,476 SUB TOTAL \$23,859,140 ENGINEER & CONTING. 20% \$4,771,828 MAINLINE TOTAL . \$28,630,968 DOES NOT INCLUDE APPROACHS OR BRIDGES OPTIONS FOR ALTERNATE B SECTION 2 APPROACH AT WINN BRANCH APPROACH RT. STA. 505+575 2,124,12 \$2,124,121 APPROACH LT. STA. 505+575 \$2,808,119 TURN LANES_(Z) \$124,966 BRIDGE \$10,947,775 CULVERT \$864,000 Blockoos

GRAND TOTAL

NO.0E8

PIKE COUNTY US 119

CONST	TRUCTION	COST ESTI	MATE	
	ALTERN	ATE B		
l co	NSTRUCTIO		3	
	TA. 506+320			
LENGTH = (2880				1
CENOTI = (2000	7 (11.7 (2.000 E)	11 - (\$44\$)	(. / (1.79# jiii.	·)
	-		-	
			UNIT	
DESCRIPTION	QUANTITY	UNIT	COST	TOTAL
EXCAVATION	6.968.541	CU. METER	\$2.61	\$18,187,892
SITUATION SIZE CROSS DRAINS	LS	LS	LS	\$447,334
MÉDIAN CROSS DRAINS	LS	LS	LS	\$101,049
MEDIAN BOXS	35	EACH	\$2,300,00	\$80,500
PERFORATED PIPE UNDERDRAIN 4"	37,796	LIN, FT.	\$5.00	\$188,980
CHANNEL LINING CLASS 4	7,440	TON	\$3.58	\$28,835
CLEARING AND GRUBBING		ACRES	\$1,200.00	\$0
SILT CHECKS	20	EA.	\$48.47	\$969
GUARDRAIL	3,200	LIN,FT,	\$8.47	\$27,104
END TREATMENTS	14	EA.	\$500.00	\$7,000
STAKING	1.789	MILE	\$45,000.00	\$80,505
AW FENCE	18,898	LIN.FT.	\$3.08	\$58,206
MAINTAIN AND CONTROL TRAFFIC	1	LUMP	\$50,000.00	\$50,000
WATER	2,000	MGAL	\$2.49	\$4,980
4" DGA "	19,112	TÓN	\$12.00	\$229,344
4° DRAINAGE BLANKET	23,805	TON	\$21.00	\$499,905
10' BASE	35.442	TÓN	\$25.37	\$899,184
1.5' BITUMINOUS CONC. SURFACE	6,717	TÓN	\$24.93	\$167,465
FULL DEPTH DGA	11,827	TON	\$11.36	\$134,355
BITUMINOUS MATERIAL FOR TACK	53	TÓN	\$238.75	\$15,041
EMULSIFIED ASPHALT RS-2	40.3	TON	\$291.18	\$11,735
BITUMINOUS SEAL AGGREGATE	336	TON	\$25.97	\$8,726
SEED AND PROTECTION		3Q.YD.	\$0.18	50
440.0004			100	
SUB TOTAL				\$21,226,878
MOBILIZATION 3%				\$635,805
DEMOBILIZATION 1.5%				\$318,403
			<u></u>	
SUB TOTAL		<u></u>		522,182,088
			Ī.,	
ENGINEER. & CONTING. 20%			ļ.	\$4,436,416
MAINLINE TOTAL *				\$26,618,505
DOES NOT INCLUDE APPROACHS (OR BRIDGES			1
				7.7
OPTIONS FOR ALTI	ERNATE B S	SECTION 3		and the second
APPROACH A				1
APPROACH LT. STA. 508+620		S	3,009,778	F 0377
APPROACH RT. STA. 508+620				
TURN LANES (2)		\$	2,134,590 124,968	
BRIDGE		\$	10,769,160	10 719 16
Jours 1	•		10,109,100	3023
GRAND TOTAL				7,736618
GUNIAN TOTAL		_		

CONSTRUCTION COST ESTIMATE ALTERNATE B CONSTRUCTION SECTION 4

1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	STA: 509+20		4	
LENGTH = (291	7 m) (2.917 kr	m) — (9570f	າ,) (1.812 ml. i)
		 	UNIT	
DESCRIPTION	QUANTITY	UNIT	COST	TOTAL
EXCAVATION	4,962,339	CU. METER	\$2.61	\$12,951,7
SITUATION SIZE CROSS DRAINS	LS	LS	LS	\$447,3
MEDIAN CROSS DRAINS	LS	LS	LS	\$101,0
MEDIAN BOXS	35	EACH	\$2,300.00	\$80,5
BOX CULVERT 511+830	LS.	LS	LS	\$244,8
PERFORATED PIPE UNDERDRAIN 4"	38,260	LIN. FT.	\$5.00	\$191,4
CHANNEL LINING CLASS 4	1,627	TON	\$3.58	\$5.8
CLEARING AND GRUBBING		ACRES	\$1,200.00	
SILT CHECKS	20	EA.	\$48.47	
SUARDRAIL	3,200	LIN.FT.	\$8.47	\$27,1
NO TREATMENTS	20	EÀ.	\$500.00	
STAKING	1,612	MILE	\$45,000.00	\$61,5
RW FENCE	19,140	ĻIN.FT.	\$3.08	\$58.9
MAINTAIN AND CONTROL TRAFFIC	1	LUMP	\$150,000.00	\$150,0
WATER	2,000	MGAL	\$2.49	\$4,9
P DGA	19,194	TON	\$12.00	
P DRAINAGE BLANKET	23,905	TON	\$21.00	\$502,0
O' BASE	35.591	TON	\$25.37	\$902,9
.5" BITUMINOUS CONC. SURFACE	6,746	TÓN	\$24.93	\$188,1
ULL DEPTH DGA	11,876	TON	\$11.36	\$134,9
SITUMINOUS MATERIAL FOR TACK	64.6	TON	\$238.75	\$15,4
MULSIFIED ASPHALT RS-2	40.8	TON	\$291,16	\$11.8
ITUMINOUS SEAL AGGREGATE	340	TON	\$25.97	\$8.6
SEED AND PROTECTION		SQ.YD.	\$0,18	
SUB TOTAL				\$16,330,65
MOBILIZATION 3%				\$489,9
EMOBILIZATION 15%				\$244,9
Cilia di Cia	 			3244)4
SUB TOTAL				\$17,065,5
NGINEER. & CONTING. 20%		20		\$3,413,1
RAND TOTAL				£20 479 E
DOES NOT INCLUDE APPROACHS	OA BAIDGES	8		\$20,478,5
				2 14-11
OPTIONS FOR ALTI				g 34
APPROACH A	1360113			N 18 (F
PPROACH LT. STA. 110+299 URN LANES (2)		\$1,67 \$124		
OPTIONS FOR ALTI	EDNATE D	SECTION 4		
APPROACH AT 119				
PPROACH LT. STA. 510+980		\$2,77	1.165	- '
PPROACH RT. STA. 510+980		\$5.28		
PPROACH LT. STA. 511+300		\$408		
VERPASS ON A		\$2,60		
URN LANES (2)		\$124		
RAND TOTAL				
MANAGE TO THE	4.1			

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CAVATION	DUAMETY	BURN 1	TOTAL	QUANTITY	BURNT UNIT	FOTAL PRICE	QUANTITY	UNUT	TOTAL	S 2	1	i
TACE	2,455,643	1978	\$5,409,229	674266	1261	\$2,761,632	7428977	15.61	16.334.410			
STATE	- 4 6.33		221.0	į		9151,356	0		F221,314		-	
10. BASE	22,803		1729,760	14628	2	545.744 646.000	30211	ā	6106,750			1
RANAGE BLANKET	165		1214,925	7360	3	S184.000	510	5	100 acc			
SA	120.1		£00°.603	6536	614	6134.946		215	1151 418			
DEPTH LGA	3,501	718	\$53,714	4014	\$14	\$36.196		215	\$77.778			
7			00.87	-		6 5,350,000	7		1,384,006			
			10 A			24,653,170			\$3,177,689			
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				APPROACH	COST	ESTIMATES A	AL TERNATE	6				
				-	119 A1		AIM					
		* SENS]								7
DISCORPTION	Ottobutty	1 100	1000	Ottobara i	DEMI S			BENT 3			BENT 4	*
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WATION	1-80070G		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		THE	THEFT		HE CE	PRICE		PRICE	PRICE
CACE			44 721 CM	A PARTIES		100g/08	24548	1561	1512,186	481730	N 82 61	\$ 1,283,415
SUMFACE	3054	153		0640	1	CON 120		+			,	•
ASE	28015	629		- Partie	2	406 P.		_	1			\$7,250
MAKE BLANKET	BIAS	25		7	1	2601100		2				
5	BEAS	113	024 64 1	7070	2 3	200 000		2	-		2	_
FULL DEPTH DGA	1120	114	5 57,736	3761	314	152 654	0,34		207.860	200		
PASS	-		(22,000)	7		1371000			ŀ		*	
TOTAL			919		<u> </u>	l						200,950
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			J	- 1			Ī					
		¥	APPROACH COST	T ESTIMATES		ALTERNATE B						
			US 119 ATF	AT RACCOON CREEK (RAC1)	CREEK IN	AC1)						
		RACI	i .		RACT			- UVO	*			
DISCRIPTION	MANUTY	E 5	TOTAL	DUMPITY (UNG	TOTAL	VIIIAMIN	Indi	TOTAL S			
		PRICE	Proce		FEECE	PARCE		PRINCE	2000			
EXCAVATION	9533048		\$ 17,364,456	2783,647	1973	1056.518	2 573 480	136	A TIG BOD			
WCE			\$ 64,300			0000			G. 3.0			
TURF ACE	5781		20/611	(25)	\$33	47,289	2723	430	A4 A17			
180	31031	3	\$ 692,062	15,369	\$72	6 494,200	19,617	22.5	27 744			
MINAGE BLANKET	12036	\$25	300,150	6.771	_		8.546	- X	217 Car			
	10446	814	101,244	5,157	_		7.00	814	ANA CO.			
DOTH DCA	36.55	916	53,970	2,752	\$10	1,529	2.ED9	514	30 704			
PASS			£ 476,000;					-				
			110,011,502			7,100,227			7,626,643			
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-		AP	APPROACH COST	T ESTIMATES		ALTERNATE B						
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4. DOA	UG-	414	207	1	1		1	1				
FULL DEPTH DGA	95	A14	19.440	†	Ť		†	+				
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			87 663 BAS	†	†			+				

Sheetz